

工程指示 / 要求簡箋(E.I.)

工程指示編號：EI / 6738 / 23

修改版次：

工程編號：J - 858

工程名稱：將軍澳日出康城

工程項目：趟門中企 鐵件拆圖 (拆圖指示)

收件人：生統 / 拆圖

發件人：Ant Yeung

日期：15/09/2023

要求提供 / 確認 事項：

- |                                    |                                     |                               |
|------------------------------------|-------------------------------------|-------------------------------|
| <input type="checkbox"/> 初步鋁料 B.M. | <input type="checkbox"/> 加工拆圖，然後生產  | <input type="checkbox"/> 尺寸表  |
| <input type="checkbox"/> 正式鋁料 B.M. | <input type="checkbox"/> 技術上資料 / 指示 | <input type="checkbox"/> 報價   |
| <input type="checkbox"/> 配件 B.M.   | <input type="checkbox"/> 樣辦或貨品說明書   | <input type="checkbox"/> 分判合約 |

內容：  
按計數和 BD 批圖 3500 高的趟門要 加力鐵 60x50

請計 BM 用量和給 拆圖

謝謝

請在 2023.9.18 前完成上列要求。

附：

- 原合約工程包                       原合約工程加 / 減賬                       新工程報價

原因：-

分發東莞各部門：

- |   |   |  |   |
|---|---|--|---|
| <input type="checkbox"/> 生產技術總監 <input type="checkbox"/> 連附件                    | <input checked="" type="checkbox"/> 技術部 <input checked="" type="checkbox"/> 連附件   | <input type="checkbox"/> 生產部 <input type="checkbox"/> 連附件    | <input type="checkbox"/> 機械設計部 <input type="checkbox"/> 連附件               |
| <input type="checkbox"/> 採購部 <input type="checkbox"/> 連附件                       | <input checked="" type="checkbox"/> 生產統籌部 <input checked="" type="checkbox"/> 連附件 | <input type="checkbox"/> 小羅 & 清 <input type="checkbox"/> 連附件 |   |
| <input checked="" type="checkbox"/> 質檢部 <input checked="" type="checkbox"/> 連附件 | <input type="checkbox"/> 會計部 <input type="checkbox"/> 連附件                         | <input type="checkbox"/> 報關組 <input type="checkbox"/> 連附件    | <input type="checkbox"/> 其他 _____ <input checked="" type="checkbox"/> 連附件 |

分發香港各部門：

- 行政部  連附件     會計部  連附件     統籌部  連附件     工程部地盤科文 積哥  連附件  
 採購部  連附件     QS 部  連附件     維修部  連附件     其他 \_\_\_\_\_  連附件

傳遞編號：

HK 2153 / 23

發件人簽署：

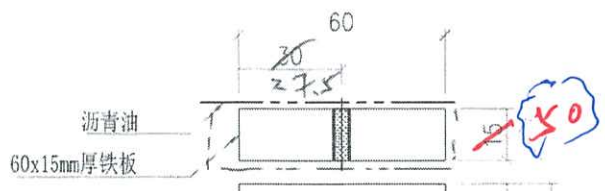
項目經理簽署：

591  
592

 美特鋁質有限公司 MIDI Aluminium Fabricator Ltd.		工程號 J858 地盘 康城11期	類別 制圖 W. E. X 復核 - 批准 -	物料號 - 2022-07-25 - -	圖號 SD-GS05 數量 - 單件重量 17.55 (KG)
修改 日期	採用工廠 <input checked="" type="checkbox"/> 地盘 材料颜色 热浸锌	图纸名称 铁料加工图 材料 S-275			

位置	QTY
T1	572
T2	624
T3	624
合计:	1820

+180  
=2000件

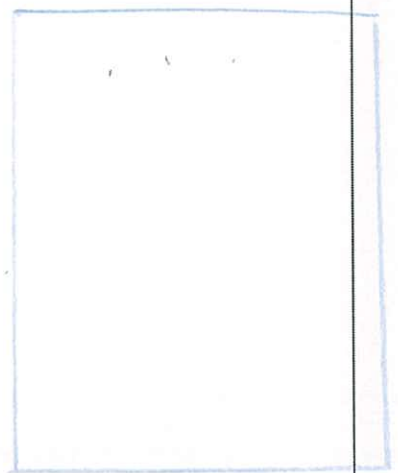
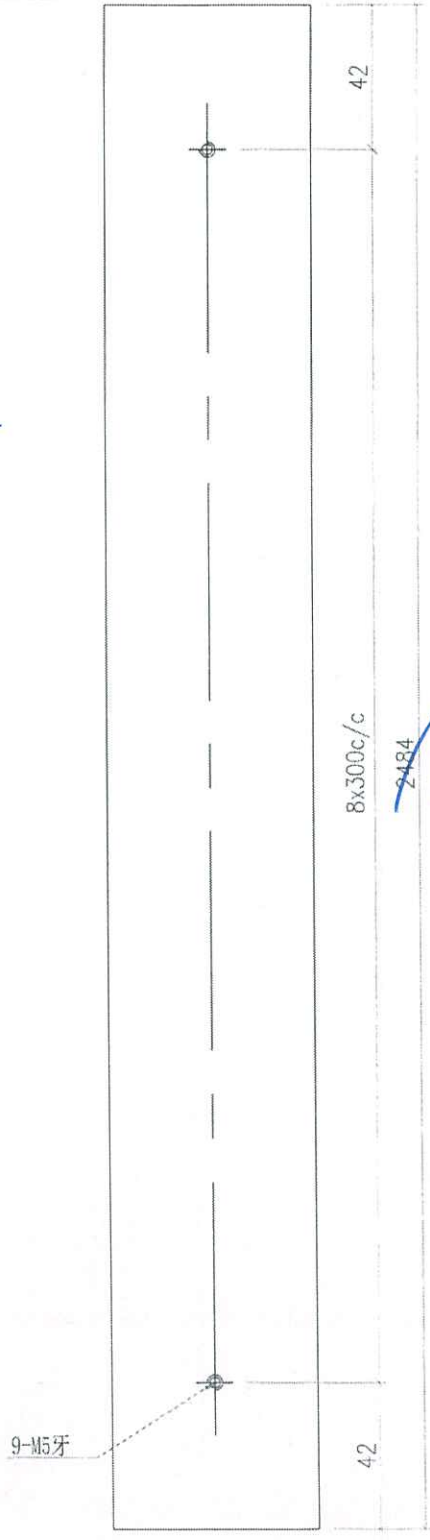
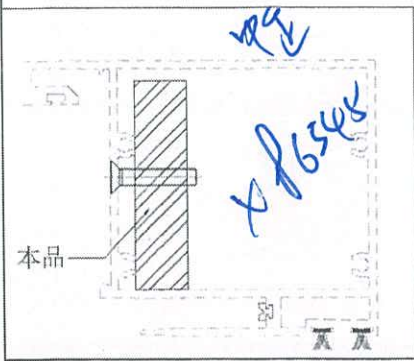


~~60x40~~  
~~60x40~~  
60x50 SU  
TYP 15x50  
SU 60x50 ←

特层  
超门中压

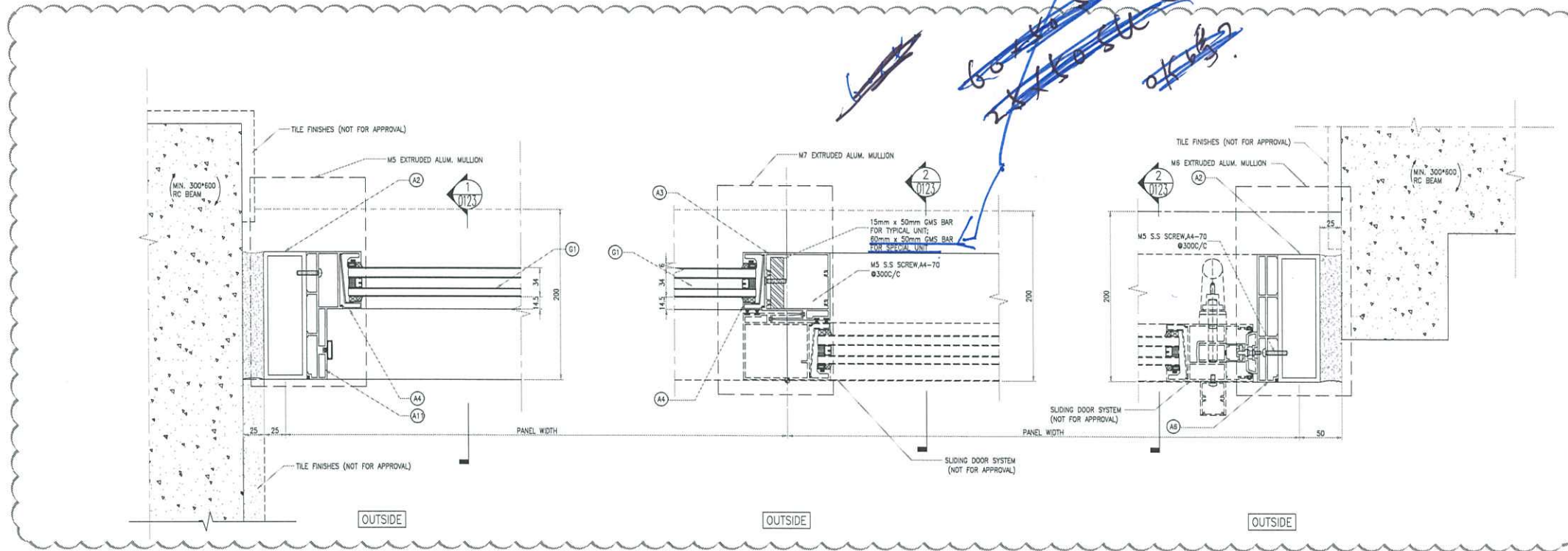
原用  
冲色, 同色用

8x300c/c  
2484  
L=



每幅門1只

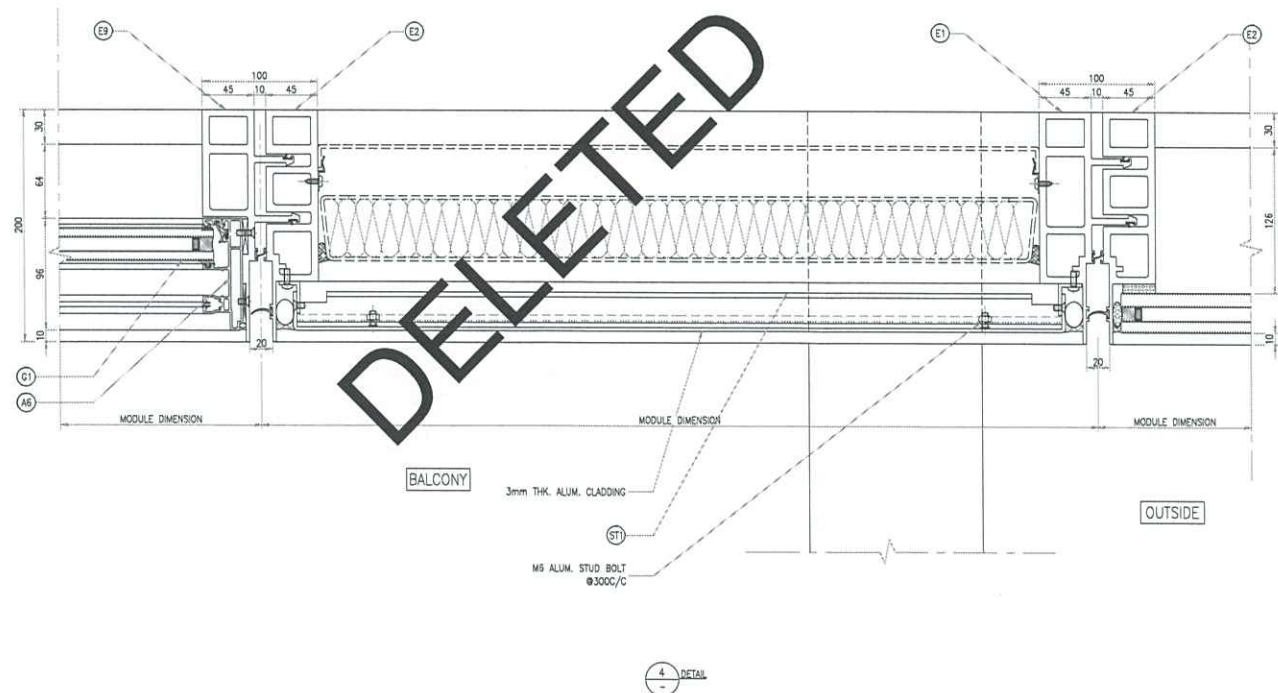
54  
 1000 1000  
 3300  
 1200 1200  
 3500  
 60x50 窗框



1 DETAIL  
 (REFER TO DWG. 2/0071-0076,  
 2/0081-0086, 2/0091-0096)

2 DETAIL  
 (REFER TO DWG. 2/0071-0076,  
 2/0081-0086, 2/0091-0096)

3 DETAIL  
 (REFER TO DWG. 2/0071-0076,  
 2/0081-0086, 2/0091-0096)



4 DETAIL

**B.D. AMENDMENT**

THE WORKS SHOWN ON THESE PLANS ARE THE PROPERTY OF  
 SUPERSTRUCTURE (CURTAIN WALL & TOWER) S.P. CO., LTD.  
 IN RESPECT OF WHICH CONSENT IS APPLIED FOR THE PURPOSE  
 OF FAST TRACK, URGENT APPLICATION UNDER REGULATION 31  
 OF THE BUILDING (ADMINISTRATIVE) REGULATIONS.

B.D. REF :

CLIENT :  
 信和置業有限公司  
 Sino Land Company Limited

ARCHITECT :  
 ronald lu & partners  
 Architects, Planners, Interior Designers  
 Ronald Lu & Partners (HK) Ltd.  
 呂元祥建築師事務所 (香港) 有限公司

MAIN CONTRACTOR :  
 Gammon

STRUCTURAL ENGINEER :  
 AECOM

FAÇADE CONSULTANT:  
 AECOM

NOTE :  
 1. ALL DIMENSIONS ARE IN mm.  
 2. ALL ELEVATIONS ARE VIEWED FROM OUTSIDE.  
 3. ALL DIMENSIONS TO BE VERIFIED ON SITE BEFORE FABRICATION.

LEGEND :  
 X1 --- DETAIL MARK NO.  
 X001 --- REFER SHEET NO.  
 1. F.F.L. --- FINISHED FLOOR LEVEL  
 2. S.F.L. --- STRUCTURAL FLOOR LEVEL  
 3. /R --- REVERSED DETAIL

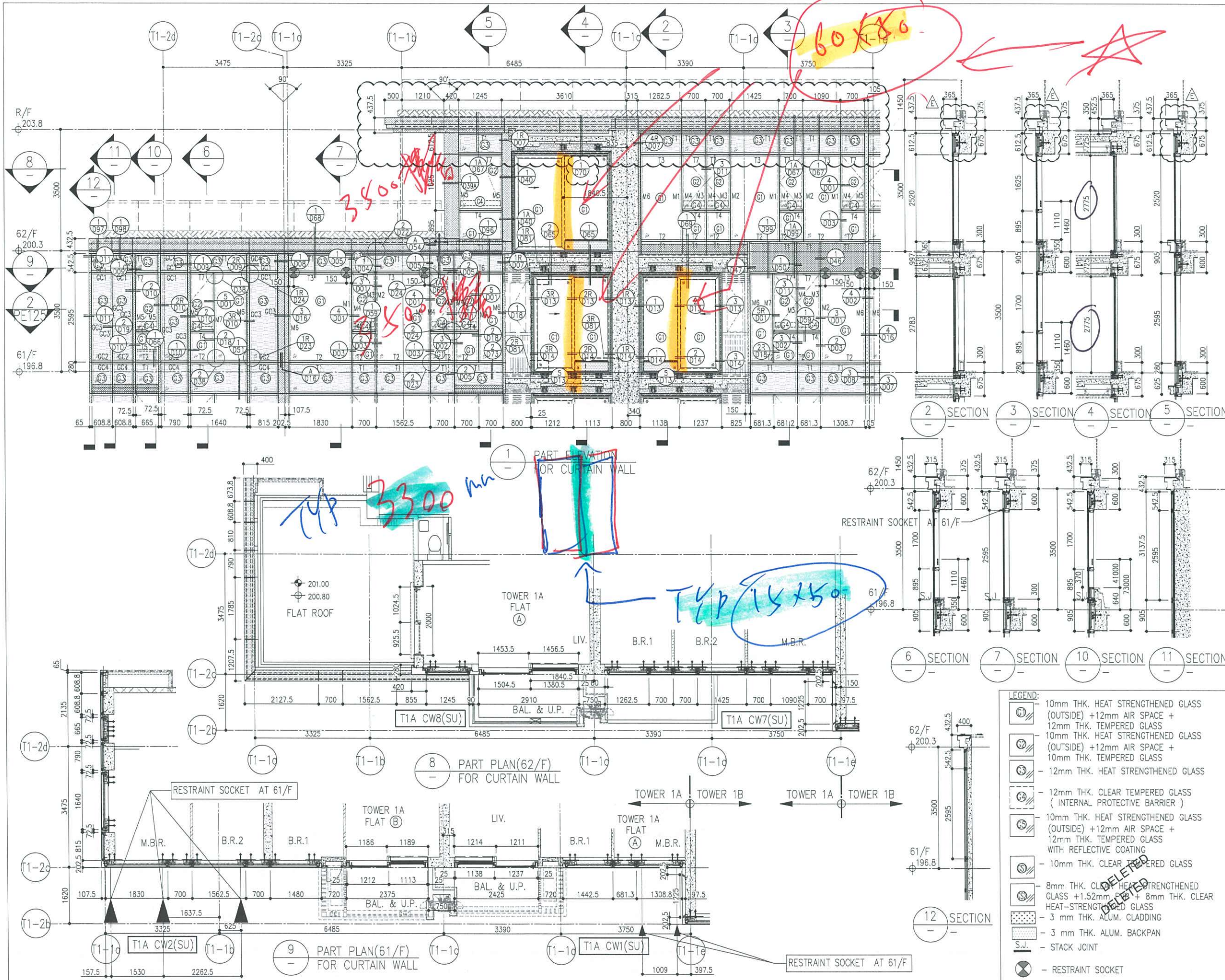
B	25.04.22	BD AMENDMENT	K
A	28.12.21	BD AMENDMENT	K
-	30.09.20	BD SUBMISSION	K
NO.	DATE	REVISED	BY

JOB NO. : J-858  
 PROJECT :  
 PROPOSED RESIDENTIAL DEVELOPMENT  
 AT TKOTL 70RP, PHASE 11, LOHAS PARK  
 TSEUNG KWAN O, N.T.

TITLE :  
 FAÇADE DETAIL

DATE : 22-OCT-21 SCALE : 1:1(A1)  
 DRAWN BY : KY CHECKED BY : YB

美特鋁質有限公司  
 MIDI ALUMINIUM FABRICATOR LTD.  
 Units 6-8, Sunray Industrial Centre, 1/F  
 610 Cha Kwo Ling Road, Kowloon  
 Tel:23489211-4 Fax:(852)27727666  
 DWG NO. : LP11/0102 REV. : B



B.D. REF :

CLIENT :  
 信和置業有限公司  
 Sino Land Company Limited

ARCHITECT :  
 ronald lu & partners  
 Ronald Lu & Partners (HK) Ltd.  
 呂元祥建築師事務所 (香港) 有限公司

MAIN CONTRACTOR :  
 Gammon

STRUCTURAL ENGINEER :  
 AECOM

FAÇADE CONSULTANT :  
 AECOM

NOTE :  
 1. ALL DIMENSIONS ARE IN mm.  
 2. ALL ELEVATIONS ARE VIEWED FROM OUTSIDE.  
 3. ALL DIMENSIONS TO BE VERIFIED ON SITE BEFORE FABRICATION.

LEGEND :  
 (X1) -- DETAIL MARK NO.  
 (X001) -- REFER SHEET NO.

1. F.F.L. -- FINISHED FLOOR LEVEL  
 2. S.F.L. -- STRUCTURAL FLOOR LEVEL  
 3. (R) -- REVERSED DETAIL

NO.	DATE	REVISION	BY
E	14/08/23	REVISED AS PER COMMENTS	LS
D	18/04/23	REVISED AS PER COMMENTS	LS
C	26/07/22	REVISED AS PER COMMENTS	LS
B	26/05/22	REVISED AS PER COMMENTS	LS
A	13/04/22	REVISED AS PER COMMENTS	LS

JOB NO. : J-858

PROJECT :  
 PROPOSED RESIDENTIAL DEVELOPMENT  
 AT TKOTL 70RP, PHASE 11, LOHAS PARK  
 TSEUNG KWAN O, N.T.

TITLE :  
 PENTHOUSE FLOOR PARTIAL  
 ELEVATION & SECTION, PLAN  
 FOR CURTAIN WALL (TOWER 1)

DATE : 12-JAN-22      SCALE : 1:50(A1)

DRAWN BY :      CHECKED BY :

美特鋁質有限公司  
 MIDI ALUMINIUM FABRICATOR LTD.  
 Units 6-8, Sunray Industrial Centre, 1/F  
 610 Che Kwo Ling Road, Kowloon  
 Tel: 23489211-4 Fax: (852) 2727666

DWG NO. : J858-CW-PE117      REV. : E



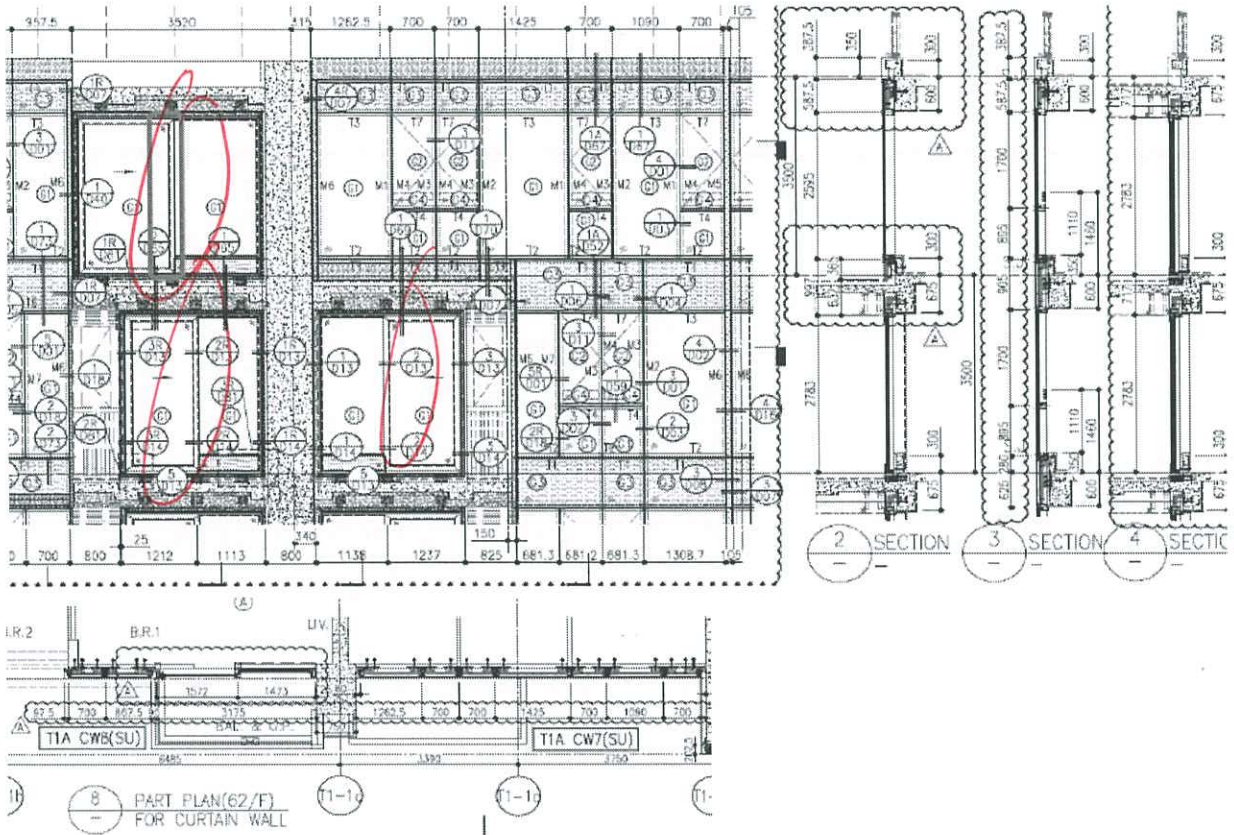
J850

7591



8.1A Check Sliding Door Middle Mullion at penthouse

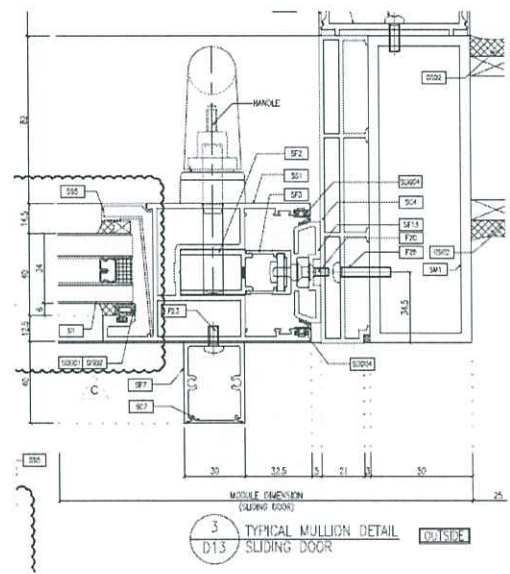
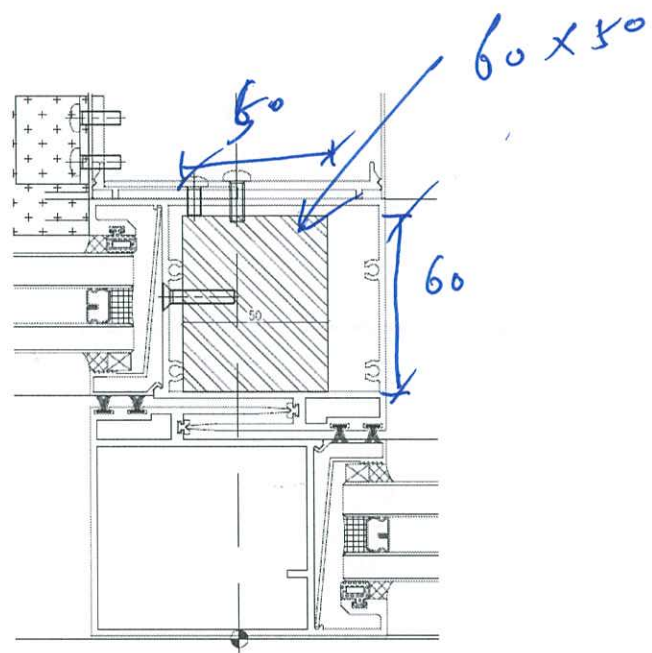
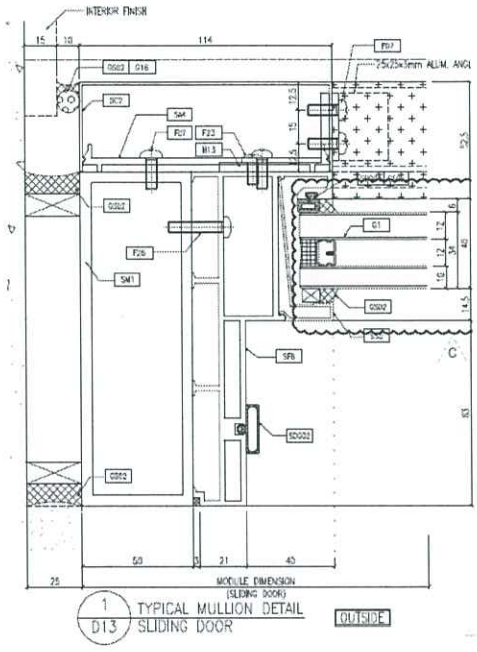
Refer to DWG: J858-CW-PE117



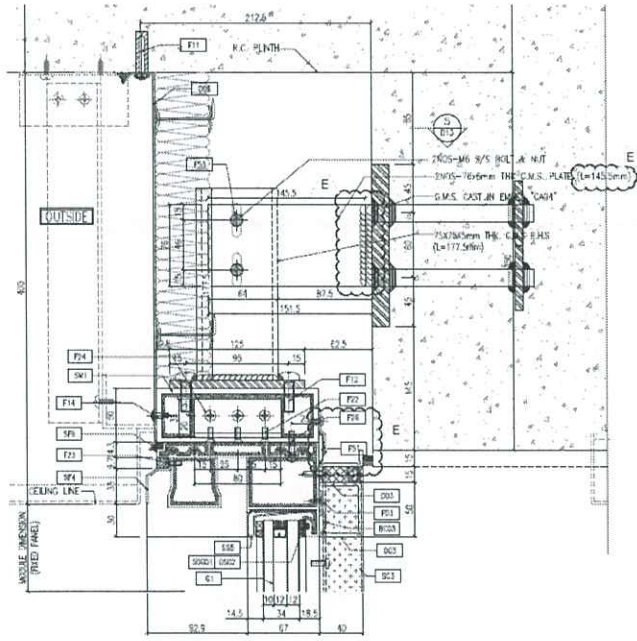
這 350 高 超 門 的 加 力 鉗

用

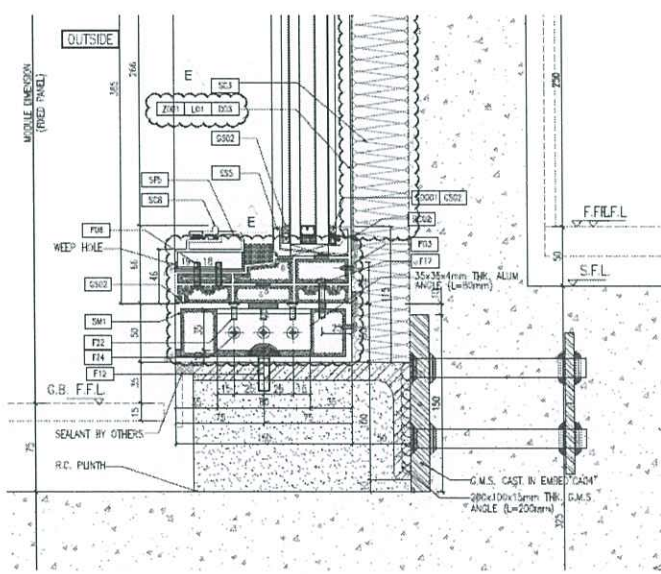
Refer to DWG: J858-CW-D13



Refer to DWG: J858-CW-D65



1 TYPICAL TRANSOM DETAIL  
D65 SLIDING DOOR



## Loading Analysis

### 1.1 Wind Load

Wind load on curtain wall  $WL := 4.57 \text{ KPa}$

### 1.2 Dead Load

Density of glass  $D_{gl} = 2650 \cdot \text{kg} \cdot \text{m}^{-3}$

Gravity density of glass  $G_{gl} := D_{gl} \cdot g$   $G_{gl} = 26 \cdot \text{KN} \cdot \text{m}^{-3}$

Glass thickness  $t_{gl} := 22 \text{ mm}$

Dead load due to glass weight  $DL := G_{gl} \cdot t_{gl} \cdot (1 + 10\%)$   $DL = 0.63 \cdot \text{KPa}$

(Increasing 10% allowing for weight of frames and other accessories)

### 1.3 Linear Load on Mullion

Span of mullion  $L_m := 2761 \text{ mm}$

Modulus dimension  $B_1 := 1572 \text{ mm}$

$B_2 := 1473 \text{ mm}$

$$a_1 := \frac{\min(L, B_1)}{2}$$

$a_1 = 786 \cdot \text{mm}$

$$a_2 := \frac{\min(L, B_2)}{2}$$

$a_2 = 736.5 \cdot \text{mm}$

Linear load due to wind  $q_1 := WL \cdot a_1$

$q_1 = 3.59 \cdot \text{N} \cdot \text{mm}^{-1}$

$$q_2 := WL \cdot a_2$$

$q_2 = 3.37 \cdot \text{N} \cdot \text{mm}^{-1}$

$$M_1 := \frac{q_1 \cdot L^2}{24} \cdot \left( 3 - 4 \cdot \frac{a_1^2}{L^2} \right)$$

$M_1 = 3.05 \cdot \text{KN} \cdot \text{m}$

$$M_2 := \frac{q_2 \cdot L^2}{24} \cdot \left( 3 - 4 \cdot \frac{a_2^2}{L^2} \right)$$

$M_2 = 2.9 \cdot \text{KN} \cdot \text{m}$

Maximum bending moment due to wind  $M_{\max} := M_1 + M_2$

$M_{\max} = 5.96 \cdot \text{KN} \cdot \text{m}$

$$V_1 := \frac{q_1 \cdot L}{2} \cdot \left( 1 - \frac{a_1}{L} \right)$$

$V_1 = 3.55 \cdot \text{KN}$

$$V_2 := \frac{q_2 \cdot L}{2} \cdot \left( 1 - \frac{a_2}{L} \right)$$

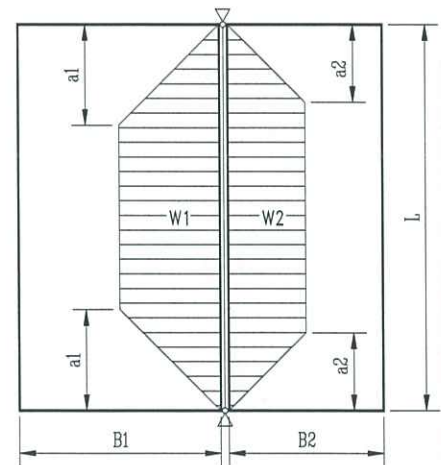
$V_2 = 3.41 \cdot \text{KN}$

Maximum shear force due to wind  $V_{\max} := V_1 + V_2$

$V_{\max} = 6.95 \cdot \text{KN}$

Axis load due to weight  $P := DL \cdot L \cdot (B_1 + B_2) \cdot 0.5$

$P = 2.64 \cdot \text{KN}$



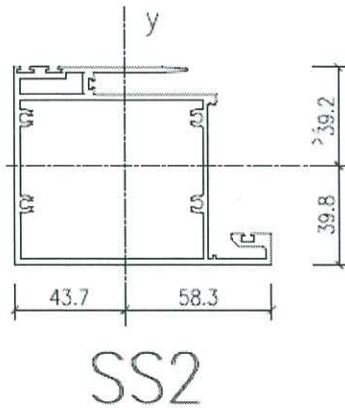
## 2.0 > Analysis Model

### 2.1 Material Properties

Alum. alloy condition	$\begin{pmatrix} p_o \\ p_a \\ p_v \end{pmatrix} := \text{Alum\_Strength}(\text{Alloy\_Condition}) \quad \text{Alloy\_Condition} = "6063-T6"$		
Limiting stresses	$p_o = 160 \cdot \text{MPa}$	Limiting stress for bending and overall yielding	
	$p_a = 175 \cdot \text{MPa}$	Limiting stress for local capacity of the section in tension or compression	
	$p_v = 95 \cdot \text{MPa}$	Limiting stress in shear	
Material properties	$E_{al} = 70000 \cdot \text{MPa}$	Young's modulus	$\nu_{al} = 0.33$ Poison's ratio
	$G_{al} := \frac{E_{al}}{2 \cdot (1 + \nu_{al})}$		$G_{al} = 26316 \cdot \text{MPa}$ Shear modulus
Shape factor	$\gamma_s := 1.2$	The assumed shape factor for closed section = 1.2 is consistent with generally accepted structural and facade engineering principles.	
Material factor	$\gamma_m := 1.2$	[BS8118 Table3.3]	
Load factor	$\gamma_f := 1.2$	[BS8118 Table3.1]	

## 2.2 Section Elastic Properties

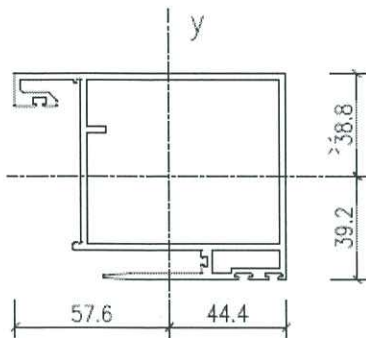
- Section Properties of SS2



MASS PROPERTIES (UNIT)	VALUES
Area (mm <sup>2</sup> ) :	1082.2
Perimeter (mm) :	937.3
Bounding Box - X (mm):	-43.7 to 58.3
Bounding Box - Y (mm):	-39.8 to 39.2
Centroid - X (mm) :	0.0
Centroid - Y (mm) :	0.0
Moments of inertia - X (mm <sup>4</sup> ) :	986468.9
Moments of inertia - Y (mm <sup>4</sup> ) :	1164232.0
Product of inertia - XY (mm <sup>4</sup> ) :	321844.5
Radii of gyration - X (mm) :	30.2
Radii of gyration - Y (mm) :	32.8
Principal moments along X-Y (mm <sup>4</sup> ) :	741458.5 along [0.8 -0.6]
Principal moments along Y-X (mm <sup>4</sup> ) :	1409242.4 along [0.6 0.8]
Elastic Modulus - Z <sub>x</sub> (mm <sup>3</sup> ):	I / y-max= 24774.6
Elastic Modulus - Z <sub>y</sub> (mm <sup>3</sup> ):	J / x-max= 19963.8
Aluminium Weight (kg/m)	2.935
Steel Weight (kg/m)	8.495

### MALE MULLION:

Section Area	$A_m := 1082.2 \text{ mm}^2$	
Moment Inertia(X-X)	$I_{x_m} := 986468.9 \text{ mm}^4$	
Moment Inertia(Y-Y)	$I_{y_m} := 1164232 \text{ mm}^4$	
Elastic Modulus(X-X)	$Z_{x_m} := 24774.6 \text{ mm}^3$	
Elastic Modulus(Y-Y)	$Z_{y_m} := 19963.8 \text{ mm}^3$	
Plastic Modulus(X-X)	$S_{x_m} := 1.2 \cdot Z_{x_m}$	$S_{x_m} = 29729.52 \cdot \text{mm}^3$
Plastic Modulus(Y-Y)	$S_{y_m} := 1.2 \cdot Z_{y_m}$	$S_{y_m} = 23956.56 \cdot \text{mm}^3$
Radii of Gyration(X-X)	$r_{x_m} := I_{x_m}^{0.5} \cdot A_m^{-0.5}$	$r_{x_m} = 30.19 \cdot \text{mm}$
Radii of Gyration(Y-Y)	$r_{y_m} := I_{y_m}^{0.5} \cdot A_m^{-0.5}$	$r_{y_m} = 32.8 \cdot \text{mm}$



SM2

MASS PROPERTIES (UNIT)	VALUES
Area (mm <sup>2</sup> ) :	1024.2
Perimeter (mm) :	874.1
Bounding Box - X (mm):	-57.6 to 44.4
Bounding Box - Y (mm):	-39.2 to 38.8
Centroid - X (mm) :	0.0
Centroid - Y (mm) :	0.0
Moments of inertia - X (mm <sup>4</sup> ) :	940697.5
Moments of inertia - Y (mm <sup>4</sup> ) :	1087216.3
Product of inertia - XY (mm <sup>4</sup> ) :	327480.9
Radii of gyration - X (mm) :	30.3
Radii of gyration - Y (mm) :	32.6
Principal moments along X-Y (mm <sup>4</sup> ) :	678381.8 along [0.8 -0.6]
Principal moments along Y-X (mm <sup>4</sup> ) :	1349532.0 along [0.6 0.8]
Elastic Modulus - Z <sub>x</sub> (mm <sup>3</sup> ):	I / y-max= 24021.3
Elastic Modulus - Z <sub>y</sub> (mm <sup>3</sup> ):	J / x-max= 18859.8
Aluminium Weight (kg/m)	2.778
Steel Weight (kg/m)	8.040

Section Size

Section Area

$$A_f := 1024.2 \text{ mm}^2$$

Moment Inertia(X-X)

$$I_{x\_f} := 940697.5 \text{ mm}^4$$

Moment Inertia(Y-Y)

$$I_{y\_f} := 1087216.3 \text{ mm}^4$$

Elastic Modulus(X-X)

$$Z_{x\_f} := 24021 \text{ mm}^3$$

Elastic Modulus(Y-Y)

$$Z_{y\_f} := 18859.8 \text{ mm}^3$$

Plastic Modulus(X-X)

$$S_{x\_f} := \gamma_s \cdot Z_{x\_f}$$

Plastic Modulus(Y-Y)

$$S_{y\_f} := \gamma_s \cdot Z_{y\_f}$$

Radii of Gyration(X-X)

$$r_{x\_f} := I_{x\_f}^{0.5} \cdot A_f^{-0.5} \quad r_{x\_f} = 30.31 \cdot \text{mm}$$

Radii of Gyration(Y-Y)

$$r_{y\_f} := I_{y\_f}^{0.5} \cdot A_f^{-0.5} \quad r_{y\_f} = 32.58 \cdot \text{mm}$$

Young's modulus

$$E_{al} = 70000 \cdot \text{MPa}$$

Depth of GMS Reinforcement	$b := 60\text{mm}$
Thk of GMS Reinforcement	$t := 50\text{mm}$
Section Area	$A_s := b \cdot t = 3000\text{mm}^2$
Moment Inertia(X-X)	$I_{x\_s} := \frac{b^3 \cdot t}{12} = 900000 \cdot \text{mm}^4$
	$Z_{x\_s} := \frac{b^2 \cdot t}{6} = 30000 \cdot \text{mm}^3$
Young's modulus	$E_{st} = 205000 \cdot \text{MPa}$

• **Section Properties of Mullion**

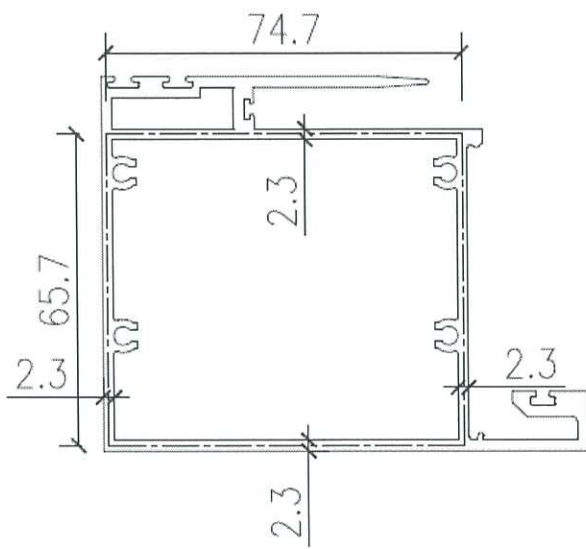
Moment Inertia(X-X)	$I_x := I_{x\_m} + I_{x\_f} + I_{x\_s} \cdot \frac{E_{st}}{E_{al}}$	$I_x = 4562880.69 \cdot \text{mm}^4$
Load share factor (male)	$\eta_m := \frac{I_{x\_m}}{I_x}$	$\eta_m = 0.22$
Load share factor (female)	$\eta_f := \frac{I_{x\_f}}{I_x}$	$\eta_f = 0.21$
Load share factor (GMS)	$\eta_s := 1 - \eta_m - \eta_f$	$\eta_s = 0.58$

**3.0 > Check Deflection**

Deflection due to wind	$\delta_1 := \frac{q_1 \cdot L^4}{240 \cdot E_{al} \cdot I_x} \left[ \frac{25}{8} - 5 \cdot \left( \frac{a_1}{L} \right)^2 + 2 \cdot \left( \frac{a_1}{L} \right)^4 \right]$	$\delta_1 = 7.44 \cdot \text{mm}$
Deflection due to wind	$\delta_2 := \frac{q_2 \cdot L^4}{240 \cdot E_{al} \cdot I_x} \left[ \frac{25}{8} - 5 \cdot \left( \frac{a_2}{L} \right)^2 + 2 \cdot \left( \frac{a_2}{L} \right)^4 \right]$	$\delta_2 = 7.09 \cdot \text{mm}$
Max.deflection	$\delta_{\max} := \delta_1 + \delta_2$	$\delta_{\max} = 14.53 \cdot \text{mm}$
The allowable deflection	$\delta_{\lim} := \min \left( \frac{L}{180}, 20\text{mm} \right)$	$\delta_{\lim} = 15.34 \cdot \text{mm}$
Check deflection	<b>HENCE (<math>\delta_{\max} &lt; \delta_{\lim}</math>) = "OK,PASS!"</b>	

#### 4.0 > Check Strength

Maximum bending moment	$M_{\max} = 5.96 \cdot \text{KN} \cdot \text{m}$
Maximum shear force	$V_{\max} = 6.95 \cdot \text{KN}$
Maximum axial force	$P = 2.64 \cdot \text{KN}$
Load share factor	$\eta_m = 0.22 \quad \eta_f = 0.21$



$$t_{\text{tub}_m} := \begin{pmatrix} 2.3 \\ 2.3 \\ 2.3 \\ 2.3 \end{pmatrix} \cdot \text{mm} \quad d_{\text{tub}_m} := \begin{pmatrix} 74.7 \\ 65.7 \\ 74.7 \\ 65.7 \end{pmatrix} \cdot \text{mm}$$

Area enclosed by mean perimeter

$$A_{\text{male}} := 74.7 \text{ mm} \cdot 65.7 \text{ mm}$$

Calculation Torsional Constant

$$J_m := \frac{4 \cdot A_{\text{male}}^2}{\sum \left( \frac{d_{\text{tub}_m}}{t_{\text{tub}_m}} \right)}$$

$$J_m = 789155.6 \cdot \text{mm}^4$$

### 4.1 Check Male Mullion

- Load Shared by Male Mullion

Load share factor  $\eta_m = 0.22$

Maximum bending moment  $M_x := \eta_m \cdot M_{max}$   $M_x = 1.29 \cdot \text{KN} \cdot \text{m}$

Maximum shear force  $F_{sy} := V_{max}$   $F_{sy} = 6.95 \cdot \text{KN}$

Maximum axial force  $P_a := \eta_m \cdot P$   $P_a = 0.57 \cdot \text{KN}$

- Section Classification

BS 8118. 1991 Table 4.3 Limiting Values of $\beta$					Where: $\epsilon = (250/p_o)^{0.5}$ $p_o = \text{Limiting Stress (N/mm}^2\text{)}$
Elements	$\beta_0$		$\beta_1$		
	Unwelded	Welded	Unwelded	Welded	
Outstand Elements	$7\epsilon$	$6\epsilon$	$6\epsilon$	$5\epsilon$	
Internal Elements	$22\epsilon$	$18\epsilon$	$18\epsilon$	$15\epsilon$	

Alloy\_Condition = "6063-T6"  $p_o = 160 \cdot \text{MPa}$   $\epsilon_m := (250 \text{MPa})^{0.5} \cdot p_o^{-0.5}$   $\epsilon = 1.25$

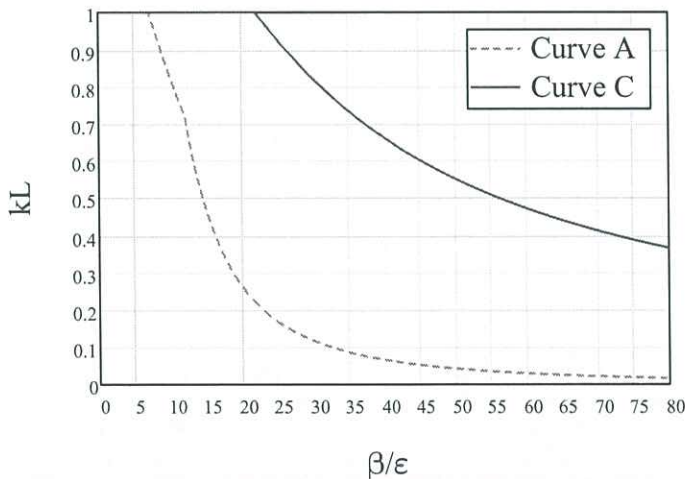
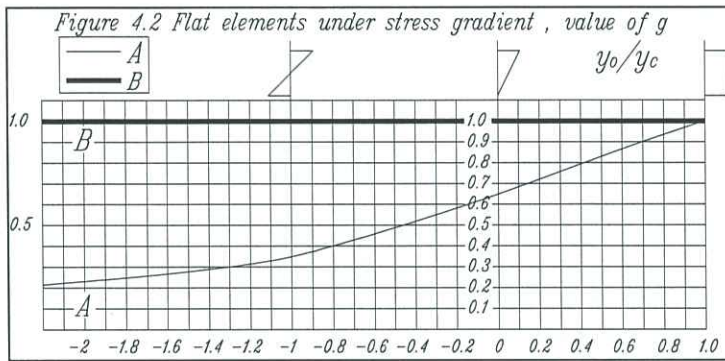


Figure 4.5 Local buckling factor  $kL$

