美 特 鋁 質 有 限 公 司
MIDI ALUMINIUM FABRICATOR LTD.

工程指示 / 要求簡箋(E.I.)

工程指示編號：**EI / 5207 / 22** 修改版次： -

工程編號 ：**J - 858** 工程名稱 ：將軍澳日出康城 11

工程項目 ：趟門 9/F 修補方案 鐵角 地盆用

收件人 ：**Maggie** 發件人 ： **Ant Yeung** 日 期 ：**14/10/2022**

☐要求提供 / ☐確認 事項 ：

( ) 初步鋁料 B.M. ( ) 加工拆圖，然後生產 ( ) 尺寸表

( ) 正式鋁料 B.M. ( ) 技術上資料 / 指示 ( ) 報價

( ) 配件 B.M. ( ) 樣辦或貨品說明書 ( ) 分判合約

內容 ：

因應 9/F 的留碼向下沉太多，原本的留碼不能使用。需打拉爆 M16

1. 附石矢報告
2. 附 Remedial 計數 19只
3. 油灘清油
4. 無焊
5. 沿水油厚度 附合 EI5136 指示 100um

請在 2022.10.20 前完成上列要求。

附： (頁

☐ 原合約工程包 ☐ 原合約工程加 / 減賬 ☐ 新工程報價

分發東莞各部門 ：

( )生產技術總監☐連附件 ( )技術部 ☐連附件 ( )生產部 ☐連附件 ( )機械設計部☐連附件

( )採購部 ☐連附件 ( )生產統籌部☐連附件 ( )小羅&清 ☐連附件

( )質檢部 ☐連附件 ( )會計部 ☐連附件 ( )報關組 ☐連附件 ( )其他 楊榮輝 ☐連附件

( )BUN ☐連附件

分發香港各部門 ：

( )行政部☐連附件 ( )會計部☐連附件 ( )統籌部☐連附件 ( )工程部地盤科文☐連附件 積哥

( )採購部☐連附件 ( )QS 部 ☐連附件 ( )維修部☐連附件 ( )其他 _____☐連附件

傳遞編號： 發件人簽署：

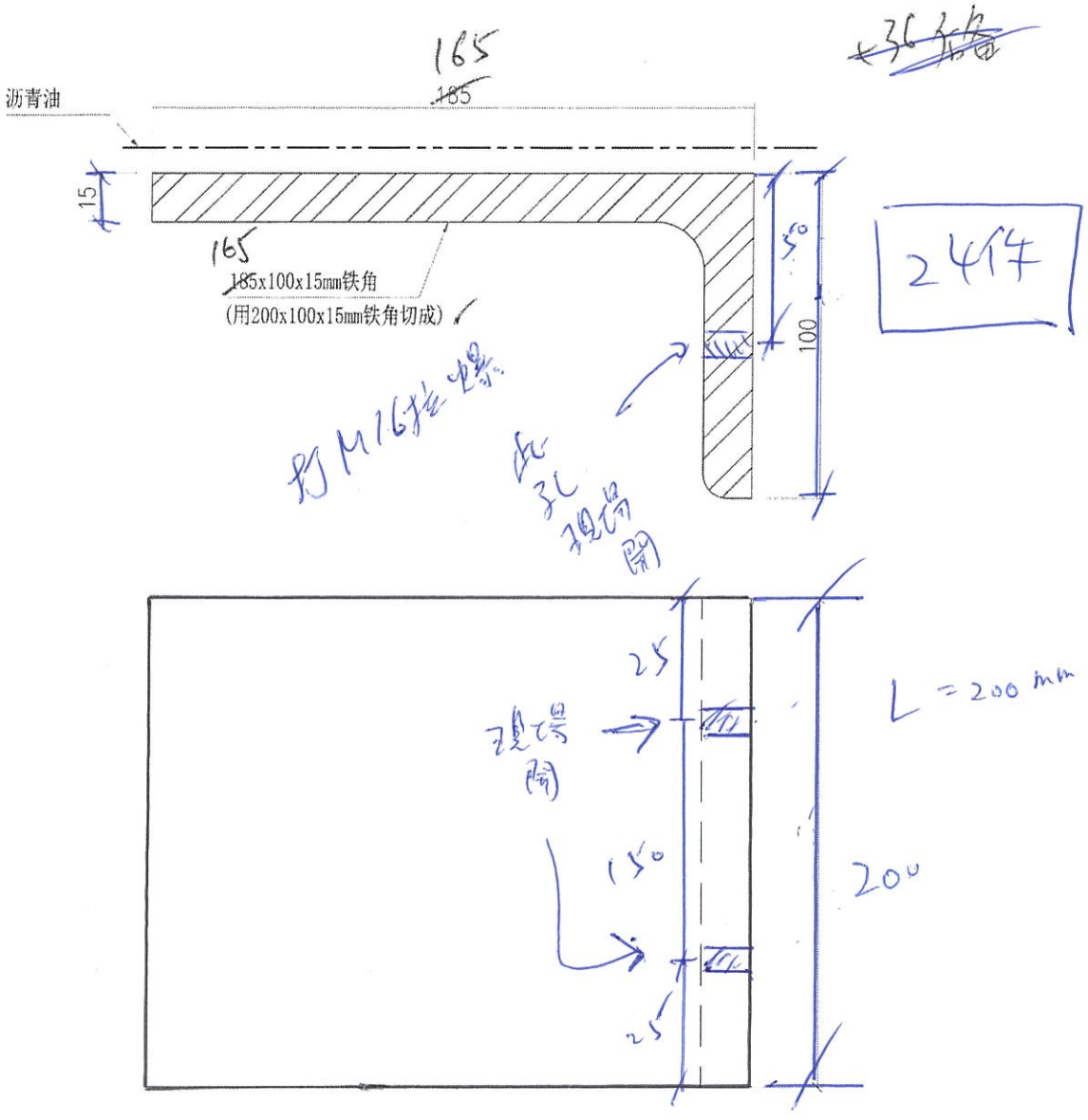
HK / 22 項目經理簽署：

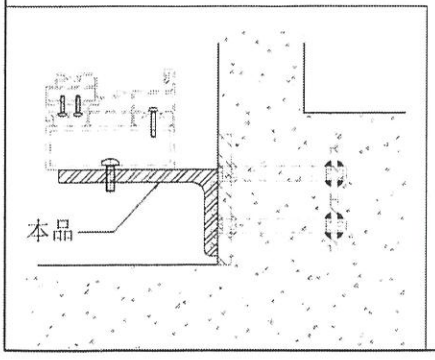
TL.5207

美特鋁質有限公司 MIDI Aluminium Fabricator Ltd.				工程號	J858	類別		物料號	- SD-RE-300L
				地盤	康城11期	制圖	W.E.X	2022-07-25	圖號 ~~SD-GS04~~
修改			採用 工廠		地盤 ✓	图纸名称	铁料加工图	復核	-	-	數量	-
日期			材料颜色	热浸锌		材料	S-275	批准	-	-	單件重量	4.81 (KG)

	位置	QTY
1794	T1	~~1716~~
1950	T2	~~1872~~
1950	T3	~~1872~~
5694	合计:	~~5460~~

~~+36件~~





門板

每幅門3只

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傳遞編號：

HK / 22

發件人簽署：

項目經理簽署：

瓦工 5207

美特鋁質有限公司 MIDI Aluminium Fabricator Ltd.											
				工程號	J858	類別		物料號	- SD-RE-300L		
				地盤	康城11期	制圖	W.E.X	2022-07-25	圖號	~~SD-GS04~~	
修改			採用 工廠		地盤 ✓	图纸名称	铁料加工图	復核	-	-	數量	-	
日期			材料顏色	热浸锌		材料	S-275	批准	-	-	單件重量	4.81	(KG)

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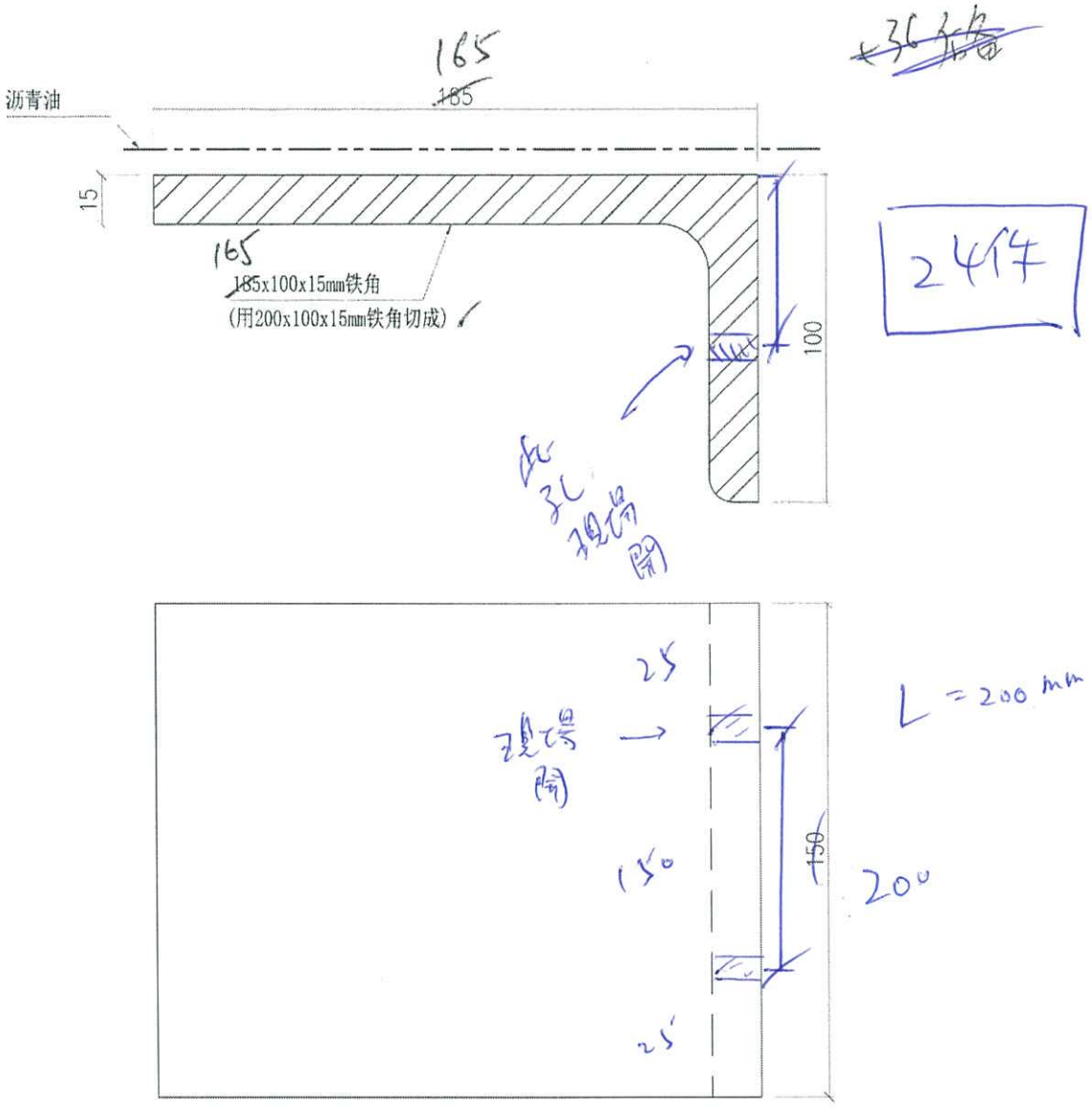
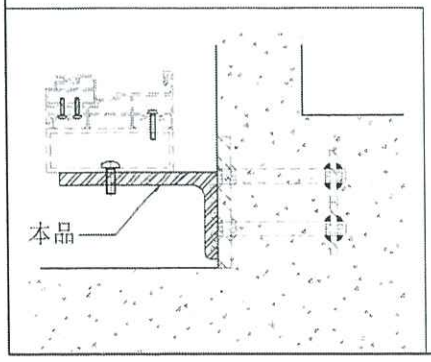




門框

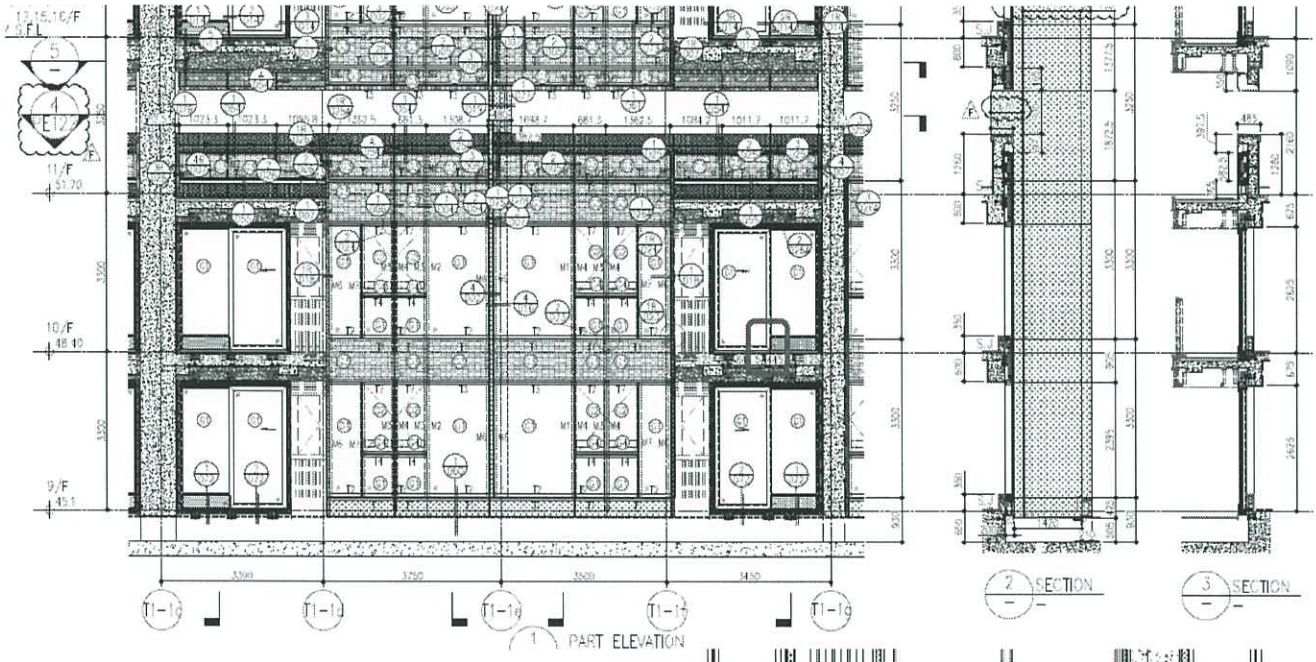
9/F

每幅門3只

PL

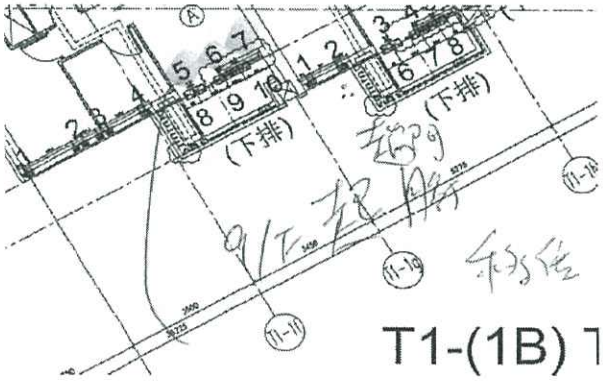
RM6- Sliding Door Bottom Fixing

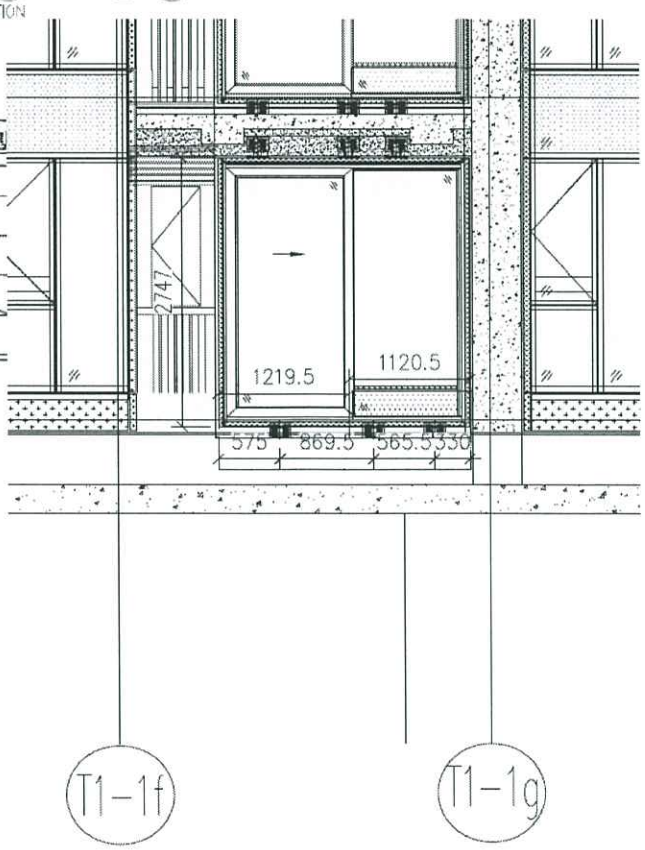
Refer to J858-CW-PE102 & D65

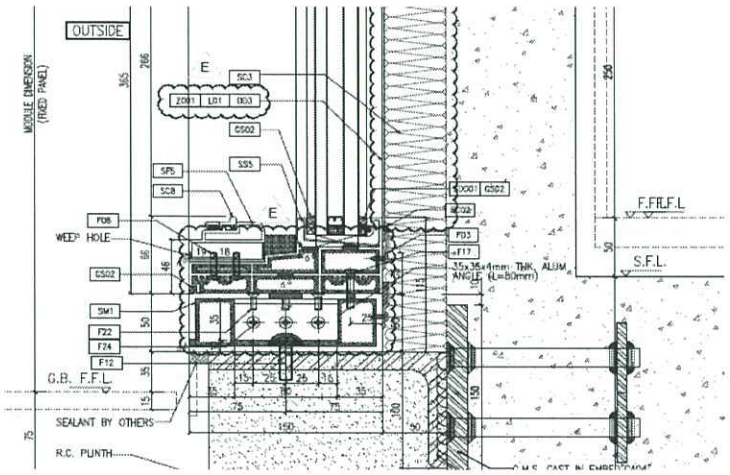


工程 J858- LP-11 日出康成11 期

位置	T 1 - (1B)	樓層 9 /F				預留碼/絲桿 類型			
單位	預留碼位編號	1	2	3	4	5	6	7	8
A	W：絲桿中至中								
	X：左/右								
	Y：上/下								
	Z：出/入								







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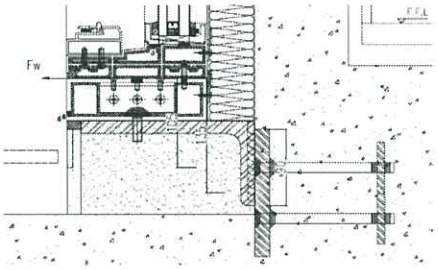
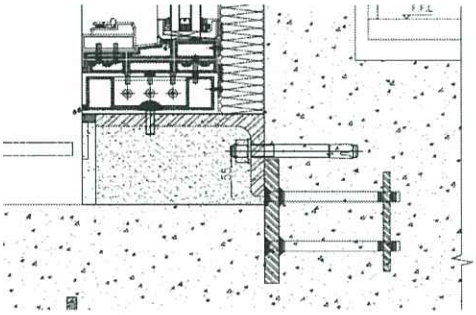


No. 5& No.6

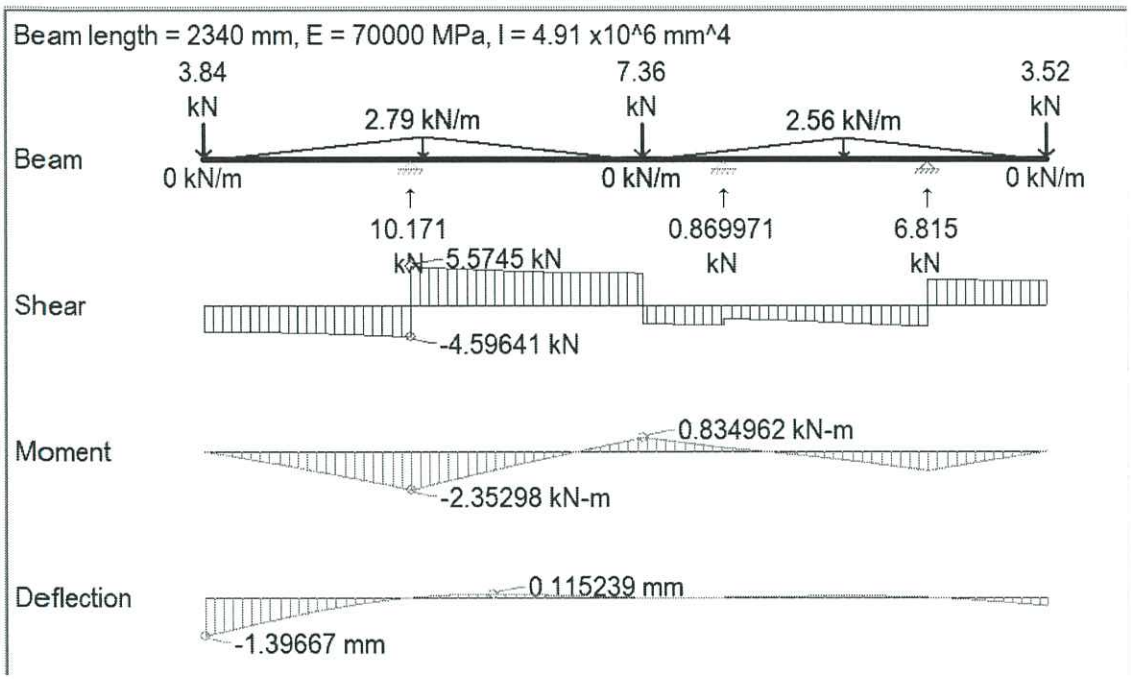


No.7

Load Analysis

Design wind load	WL	=	4.57	Kpa
Load width	B1	=	1219.5	mm
	B2	=	1120.5	mm
Effective width	B	=	(B1+B2)/2	
		=	1170.0	mm
Height of glass	H	=	2750	mm (support to support)
Liner load	q1	=	WL*min(B1/2, H/2)	
		=	2.79	KN/m
	q2	=	WL*min(B2/2, H/2)	
		=	2.56	KN/m
Concentrated load from mullion	F1	=	q1*H/2	
		=	3.83	KN
	F2	=	q2*H/2	
		=	3.520	KN
Concentrated at centre	Fc	=	F1+F2	
		=	7.352	KN

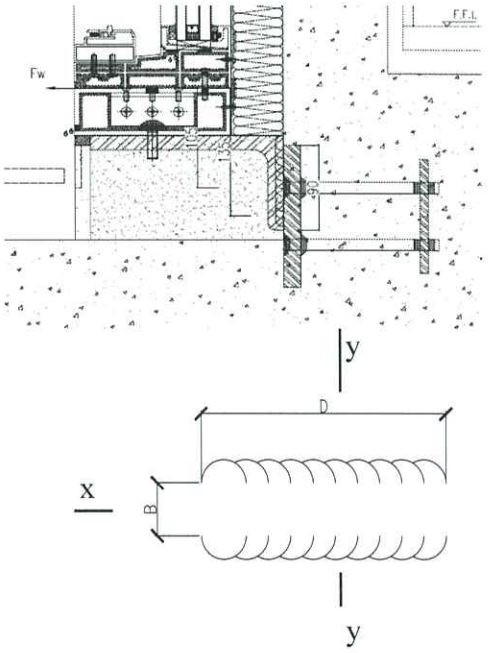
Dead load of glass transfers to backfill concrete directly, so no need to consider dead load.



Max. def	δ	= 1.40	mm	
Allowable def	δa	= 575*2/180		
		= 6.39	mm	
		> δ		O.K.

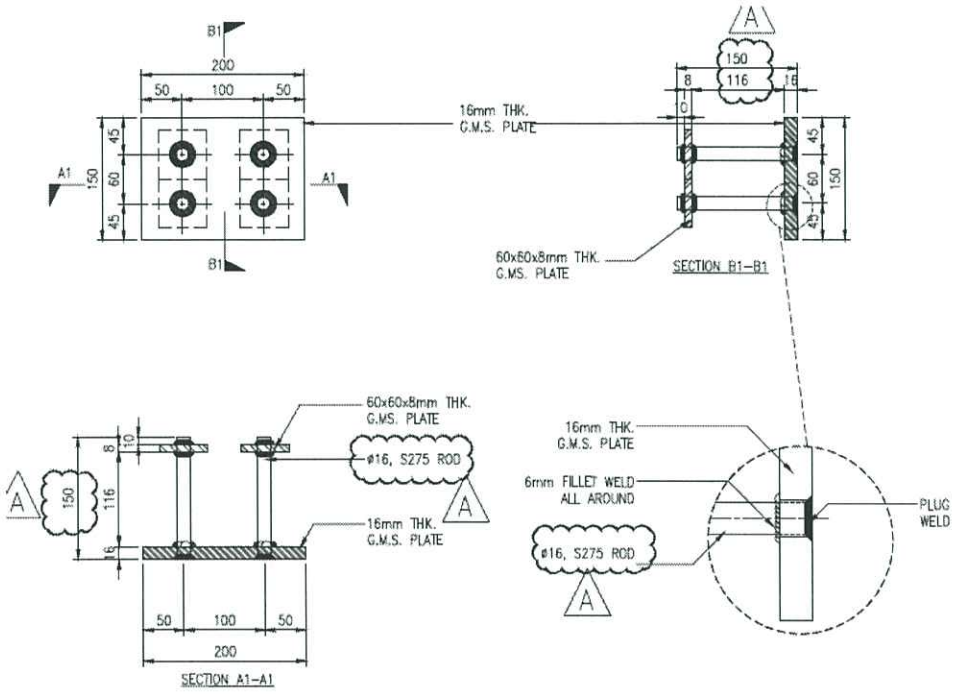
1) Check No.5 & No.6 with 50mm downward

Check 10mm thk fillet weld @ 2 side

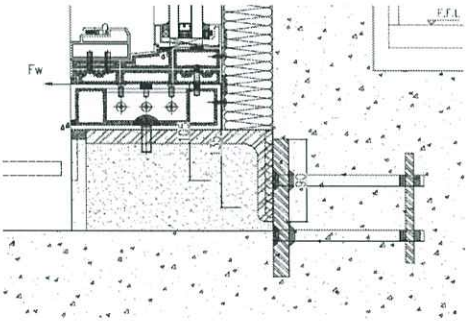


Reaction	Fw	=	10.20	KN
Ecc.	e	=	105	mm
Ecc. Moment	My	=	Fw*e	
		=	1.07	KNm
Weld thickness	t_w	=	10.0	mm
Weld size	D	=	90	mm
	B	=	150	mm
Effective weld length	ΣL	=	2D-2*tw	
		=	160	mm
Moment of inertia	I_x	=	$D * (0.5B)^2$	
		=	506250.0	mm^3
	I_y	=	$\frac{2D^3}{12}$	
		=	121500.0	mm^3
Section modulus	Z_x	=	$\frac{2I_x}{B}$	
		=	6750.0	mm^2
	Z_y	=	$\frac{2I_y}{D}$	
		=	2700.0	mm^2
Weld strength	P_w	=	220	MPa
Tensile / shear capacity	F_{vc}	=	$0.707 * P_w * t_w * \Sigma L$	
		=	248.9	kN
Moment capacity	M_{cx}	=	$0.707 * P_w * t_w * Z_x$	
		=	10.50	KNm
	M_{cy}	=	$0.707 * P_w * t_w * Z_y$	
		=	4.20	KNm
Welding Check	k	=	$\frac{1.4*Fw}{F_{vc}} + \frac{1.4*My}{Mcy}$	
		=	0.41	
		<	1	O.K.

Check cast in Embed CA04



Shear force	Fw	=	10.20	KN
Ecc.	e	=	135	mm
RC Tolerance	et	=	25	mm
Ecc. Moment	Mx	=	Fw*e	
		=	1.38	KNm
	T	=	Fw*et	
		=	0.26	KNm



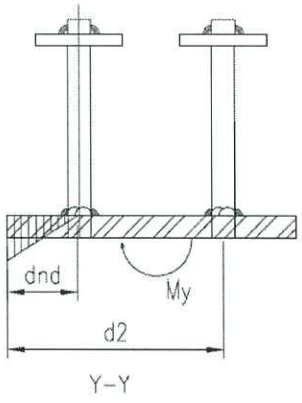
a) Check the RC Compression

Consider bending on X-X axis

Bearing strength of RC	pc	=	0.4*45MPa	
		=	18.0	Mpa
Modular ratio=Est/Erc	mo	=	15	
Design strength of Rod	py	=	275	Mpa
Tension rebar to edge plate	d1	=	105	mm
	dn1	=	$(\frac{pc*mo}{pc*mo+py})*d1$	
		=	52.02	mm
Lever arm to moment	la1	=	d1-dn1/3	
		=	87.66	mm
Tension force of per rod	T1	=	C1	= Mx/la1
		=	15.7	KN

Consider bending on Y-Y axis

Tension rebar to edge plate	d2	=	150	mm
	dn2	=	$(\frac{pc*mo}{pc*m+py})*d2$	
		=	74.31	mm
Lever arm to moment	la2	=	d2-dn2/3	
		=	125.23	mm
Tension force of per rod	T2	=	C2	= My/la2
		=	2.0	KN
Total Tension force	Ftotal	=	T1+T2	
		=	17.7	KN
Total Factored Compression force	C	=	(C1+C2)*1.4	
		=	24.8	KN
Embed plate size	b	=	200	mm
	d	=	150	mm
Compression stress of concrete	fc	=	2*C/dn1*b	
		=	4.78	Mpa
		<	fcc	= 0.6*45MPa
				= 27.0 Mpa O.K.

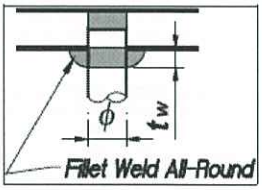


b) Check 4nos ψ16mm GMS Rod

Factored Tension force per rod	Ft	= 1.4*Fw/4+C/2		
		= 15.99	KN	
Rod spacing	s	= 100	mm	
Shear force per rod	Fs	$= \frac{1.4*T}{s*2}$		
		= 1.79	KN	
Diameter of bar:	ψ	= 16	mm	
Area	A	= $\psi^2*\pi/4$		
		= 201	mm^2	
Shear Capacity:	Ps	= 0.6*py*A		
		= 33.2	KN	
		> Fs		
		= 1.8	KN	
Check Shear	k	= Fs/Ps		
		= 0.05		
		< 1.00		O.K.
Tension Capacity:	Pt	= py*A		
		= 55.3	KN	
		> Ft		
		= 16.0	KN	
Check shear	k	= Ft/Pt		
		= 0.29		
		< 1.0		O.K.
Combined Check	k	$= \frac{Fs}{Fcs} + \frac{Ft}{Fct}$		
		= 0.34		
		< 1.40		O.K.

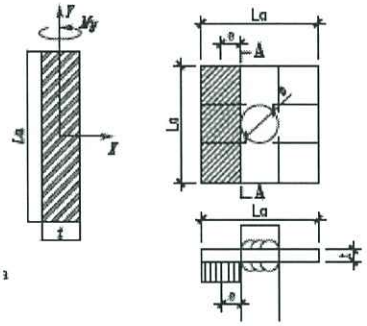
c) Check the 6mm fillet weld to fix the bar to embed

Weld thickness	tw	=	6	mm
Diameter of bar:	ψ	=	16	mm
Weld effective thickness	te	=	$tw/\sqrt{2}$	
		=	4.24	mm
Weld Length	Lw	=	ψ*π	
		=	50.2	mm
Tension stress	Prt	=	Ft/Lw	
		=	318	N/m
Shear stress	Prv	=	Fs/Lw	
		=	35.5	N/m
Resultant force	Pr	=	$(Prt^2+Prv^2)^{0.5}$	
		=	320	KN/m
Design weld strength	pw	=	220	Mpa
Weld capacity	Wc	=	0.707*tw*pw	
		=	933	KN/m
Check weld strength	k	=	Pr/Wc	
		=	0.34	
		<	1.00	O.K.



d) Check 8mm thk GMS Washer

Shear Force	Fs	=	3*Ft/8	
		=	6.00	KN
Length of section	La	=	60	mm
Ecc	e	=	(La-ψ)/4	
		=	11.00	mm
Ecc. Moment	Me	=	Fs*e	
		=	0.066	Knm



Section Properties

Section width	b	=	60	mm
Thickness	t	=	8	mm
Area	A	=	b*t	
		=	480	mm^2
Elastic Modulus	Zy	=	$b*t^2/6$	
		=	640.00	mm^3
Design strength of S275	py	=	275	Mpa
Shear Capacity	Fcs	=	0.6*py*A	
		=	79.20	KN
Moment capacity	Mcy	=	py*Zy	
		=	0.18	KNm

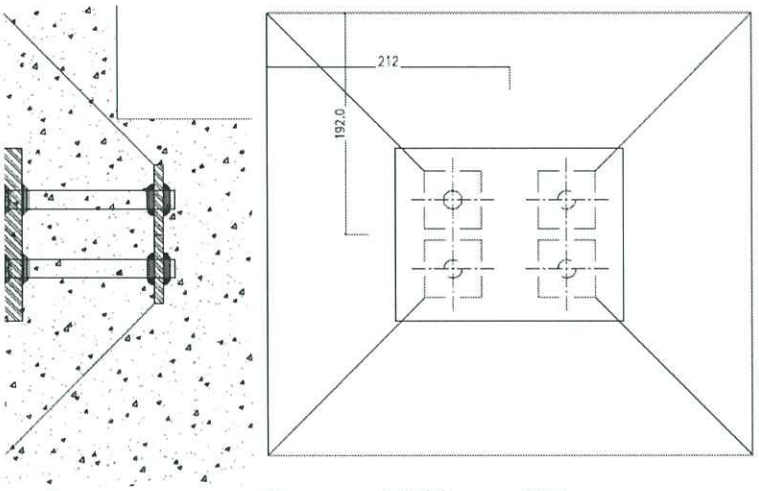
Combined check

$$k = \frac{Me}{Mcy} + \frac{Fs}{Fcs}$$

$= 0.45$

< 1.0 O.K.

e) Check Pull out resistance



Max. tension force per bar	Ft	=	15.99	KN
Total length of bar	Leff	=	132	mm
Conservative take	$\frac{100As}{P*L}$	=	0.15	
Concrete strength	fcu	=	45	Mpa
Design concrete shear strength	vc	=	$\frac{0.79}{1.25} *(\frac{100As}{P*L})^{1/3} *(\frac{400}{Leff})^{1/4} *(\frac{fcu}{25})^{1/3}$	
		=	0.54	Mpa
Washer Area	Aw	=	60mm*60mm	
Plan Area	Ap	=	192mm*212mm	
Total Area	Acp	=	Ap-Aw	
		=	44304	mm^2
Shear Cone Capacity	Fcp	=	Vc*Acp/sin(45deg)	
		=	33.8	KN
Check cone pull out	k	=	Ft/Fcp	
		=	0.47	
		<	1.0	O.K.

f) Check 15mm thk GMS Embed Plate

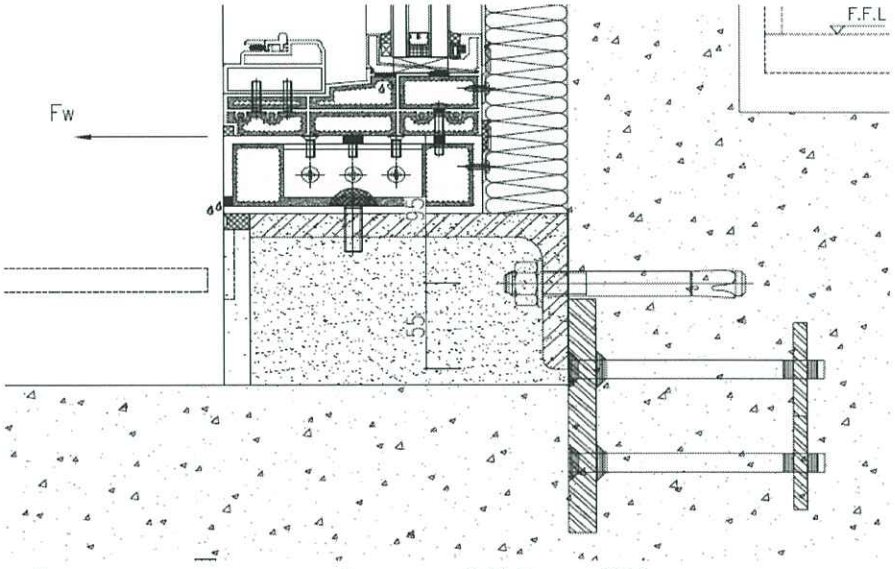
Section Properties

Length of section	b	= 200	mm	
Thickness	t	= 15	mm	
Elastic Modulus	Zy	= $b*t^2/6$		
		= 7500	mm^3	
Area	A	= b*t		
		= 3000	mm^2	
Design bending strength of S275	py	= 275	Mpa	
Design shear strength of S275	pv	= 165	Mpa	
<u>Check Shear</u>				
Shear capacity	Fcs	= pv*A		
		= 495	KN	
		> 1.4*Fw	= 14.3 KN	
Check Shear	k	= 1.4*Fd/Fcs		
		= 0.029	Low Shear Mode	
		< 1.0		O.K.
<u>Check Bending Capacity</u>				
Moment capacity	Mcy	= py*Zy		
		= 2.1	KNm	
		> 1.4*Mx	= 1.93 KNm	
Check Bending	k	= 1.4*Mx/Mcy		
		= 0.93		
		< 1.0		O.K.

g) Check the bonding stress

Tension froce per rod	Ft	=	15.99	KN
Diameter of bar:	ψ	=	16	mm
Effective anchor length	Leff	=	150	mm
Bond stress	fb	=	$\frac{Ft}{\psi*\pi*Leff}$	
		=	2.12	Mpa
Concrete Grade	fcu	=	45	Mpa
Design Ultimate bond stress	fbu	=	$0.65*(fcu)^{0.5}$	
		=	4.36	Mpa
Check Bonding	k	=	fb/fbu	
		=	0.49	
		<	1.0	O.K.

2) Check No. 7 remedial 2nos M16 HST3-R Anchor



Reaction at no.7	R	=	6.815	KN
Facctored Tension	Ft	=	2*R	
		=	13.63	KN
Ecc.	e	=	95	mm
Ecc. Moment	Me	=	Ft*e	
		=	1.29	KNm

Company:		Page:	14
Address:		Specifier:	
Phone I Fax:	|	E-Mail:	
Design:	Concrete - Oct 9, 2022	Date:	12/10/2022
Fastening point:			

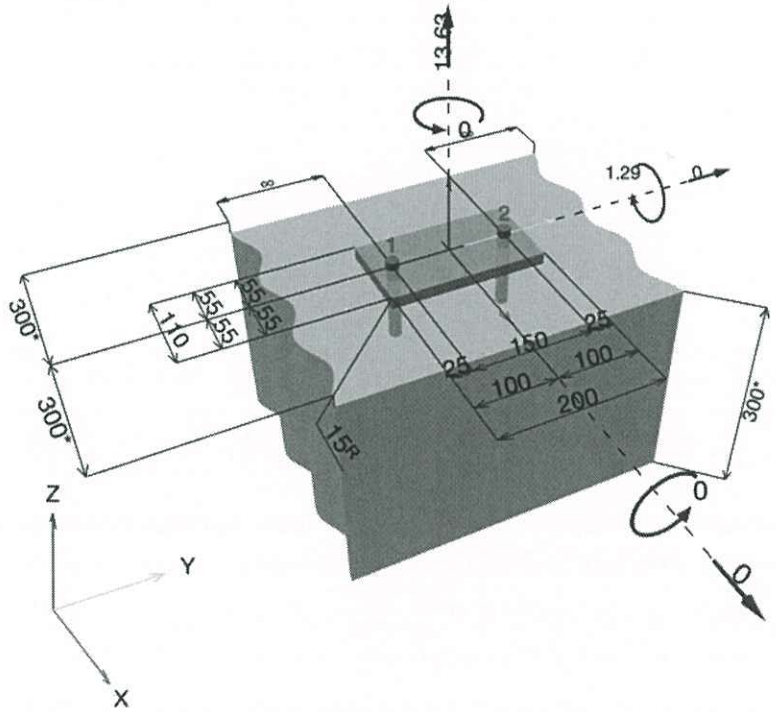
Specifier's comments:

1 Input data

Anchor type and diameter:	**HST3-R M16 hef2**
Return period (service life in years):	50
Item number:	2105876 HST3-R M16x135 35/15
Effective embedment depth:	h_{ef} = 85.0 mm, h_{nom} = 98.0 mm
Material:	A4
Evaluation Service Report:	ETA-98/0001
Issued I Valid:	13/7/2020 | -
Proof:	Design Method ETAG (No. 001 Annex C/2010)
Stand-off installation:	e_b = 0.0 mm (no stand-off); t = 15.0 mm
Anchor plate[R]:	l_x x l_y x t = 110.0 mm x 200.0 mm x 15.0 mm; (Recommended plate thickness: not calculated)
Profile:	no profile
Base material:	cracked concrete, C35/45, $f_{c,cube}$ = 45.00 N/mm^2; h = 300.0 mm
Installation:	**hammer drilled hole, Installation condition: Dry**
Reinforcement:	no reinforcement or reinforcement spacing >= 150 mm (any Ø) or >= 100 mm (Ø <= 10 mm) no longitudinal edge reinforcement

[R] - The anchor calculation is based on a rigid anchor plate assumption.

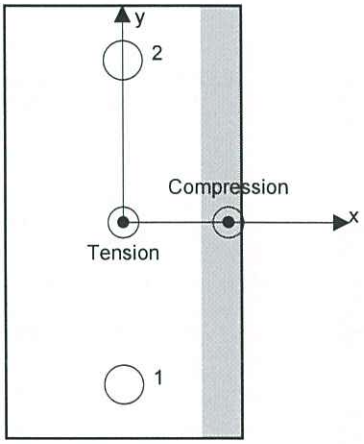
Geometry [mm] & Loading [kN, kNm]



1.1 Load combination

Case	Description	Forces [kN] / Moments [kNm]	Seismic	Fire	Max. Util. Anchor [%]
1	Combination 1	N = 13.630; V_x = 0.000; V_y = 0.000; M_x = 0.000; M_y = 1.290; M_z = 0.000;	no	no	100

2 Load case/Resulting anchor forces



Anchor reactions [kN]
Tension force: (+Tension, -Compression)

Anchor	Tension force	Shear force	Shear force x	Shear force y
1	20.005	0.000	0.000	0.000
2	20.005	0.000	0.000	0.000

max. concrete compressive strain: 0.48 [‰]
max. concrete compressive stress: 14.42 [N/mm^2]
resulting tension force in (x/y)=(0.0/0.0): 40.010 [kN]
resulting compression force in (x/y)=(48.9/0.0): 26.380 [kN]

Anchor forces are calculated based on the assumption of a rigid anchor plate.

Input data and results must be checked for conformity with the existing conditions and for plausibility!
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Company:		Page:	16
Address:		Specifier:	
Phone I Fax:	|	E-Mail:	
Design:	Concrete - Oct 9, 2022	Date:	12/10/2022
Fastening point:			

3 Tension load (ETAG, Annex C, Section 5.2.2)

	Load [kN]	Capacity [kN]	Utilization β_N [%]	Status
Steel Strength*	20.005	49.571	41	OK
Pullout Strength*	20.005	24.150	83	OK
Concrete Breakout Failure**	40.010	40.077	100	OK
Splitting failure**	N/A	N/A	N/A	N/A

* highest loaded anchor **anchor group (anchors in tension)

3.1 Steel Strength

$$N_{Sd} \leq N_{Rd,s} = \frac{N_{Rk,s}}{\gamma_{M,s}}$$ ETAG 001 Annex C, Table 5.2.2.1

$N_{Rk,s}$ [kN]	$\gamma_{M,s}$	$N_{Rd,s}$ [kN]	N_{Sd} [kN]
69.400	1.400	49.571	20.005

3.2 Pullout Strength

$$N_{Sd} \leq N_{Rd,p} = \frac{\psi_c \cdot N_{Rk,p}}{\gamma_{M,p}}$$ ETAG 001 Annex C, Table 5.2.2.1

$N_{Rk,p}$ [kN]	ψ_c	$\gamma_{M,p}$	$N_{Rd,p}$ [kN]	N_{Sd} [kN]
27.000	1.342	1.500	24.150	20.005

Input data and results must be checked for conformity with the existing conditions and for plausibility!
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3.3 Concrete Breakout Failure

$N_{Sd} \le N_{Rd,c} = \frac{N_{Rk,c}}{\gamma_{M,c}}$ ETAG 001 Annex C, Table 5.2.2.1

$N_{Rk,c} = N^0_{Rk,c} \cdot \frac{A_{c,N}}{A^0_{c,N}} \cdot \Psi_{s,N} \cdot \Psi_{re,N} \cdot \Psi_{ec1,N} \cdot \Psi_{ec2,N}$ ETAG 001 Annex C, Eq. (5.2)

$N^0_{Rk,c} = k_1 \cdot \sqrt{f_{ck,cube}} \cdot h_{ef}^{1,5}$ ETAG 001 Annex C, Eq. (5.2a)

$A^0_{c,N} = s_{cr,N} \cdot s_{cr,N}$ ETAG 001 Annex C, Eq. (5.2b)

$\Psi_{s,N} = 0.7 + 0.3 \cdot \frac{c}{c_{cr,N}} \le 1.00$ ETAG 001 Annex C, Eq. (5.2c)

$\Psi_{re,N} = 0.5 + \frac{h_{ef}}{200} \le 1.00$ ETAG 001 Annex C, Eq. (5.2d)

$\Psi_{ec1,N} = \frac{1}{1 + \frac{2 \cdot e_{c1,N}}{s_{cr,N}}} \le 1.00$ ETAG 001 Annex C, Eq. (5.2e)

$\Psi_{ec2,N} = \frac{1}{1 + \frac{2 \cdot e_{c2,N}}{s_{cr,N}}} \le 1.00$ ETAG 001 Annex C, Eq. (5.2e)

$A_{c,N}$ [mm²]	$A^0_{c,N}$ [mm²]	$c_{cr,N}$ [mm]	$s_{cr,N}$ [mm]
103,275	65,025	127.5	255.0

$e_{c1,N}$ [mm]	$\Psi_{ec1,N}$	$e_{c2,N}$ [mm]	$\Psi_{ec2,N}$	$\Psi_{s,N}$	$\Psi_{re,N}$
0.0	1.000	0.0	1.000	1.000	1.000

k_1	$N^0_{Rk,c}$ [kN]	$\gamma_{M,c}$	$N_{Rd,c}$ [kN]	N_{Sd} [kN]
7.200	37.850	1.500	40.077	40.010

Group anchor ID
1, 2

Input data and results must be checked for conformity with the existing conditions and for plausibility!
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4 Shear load (ETAG, Annex C, Section 5.2.3)

	Load [kN]	Capacity [kN]	Utilization β_V [%]	Status
Steel Strength (without lever arm)*	N/A	N/A	N/A	N/A
Steel failure (with lever arm)*	N/A	N/A	N/A	N/A
Pryout Strength*	N/A	N/A	N/A	N/A
Concrete edge failure in direction **	N/A	N/A	N/A	N/A

* highest loaded anchor **anchor group (relevant anchors)

5 Displacements (highest loaded anchor)

Short term loading:

N_{Sk} = 14.819 [kN] δ_N = 1.9906 [mm]

V_{Sk} = 0.000 [kN] δ_V = 0.0000 [mm]

δ_{NV} = 1.9906 [mm]

Long term loading:

N_{Sk} = 14.819 [kN] δ_N = 1.8800 [mm]

V_{Sk} = 0.000 [kN] δ_V = 0.0000 [mm]

δ_{NV} = 1.8800 [mm]

Comments: Tension displacements are valid with half of the required installation torque moment for uncracked concrete! Shear displacements are valid without friction between the concrete and the anchor plate! The gap due to the drilled hole and clearance hole tolerances are not included in this calculation!

The acceptable anchor displacements depend on the fastened construction and must be defined by the designer!

6 Warnings

- The anchor design methods in PROFIS Engineering require rigid anchor plates per current regulations (AS 5216:2021, ETAG 001/Annex C, EOTA TR029 etc.). This means load re-distribution on the anchors due to elastic deformations of the anchor plate are not considered - the anchor plate is assumed to be sufficiently stiff, in order not to be deformed when subjected to the design loading. PROFIS Engineering calculates the minimum required anchor plate thickness with CBFEM to limit the stress of the anchor plate based on the assumptions explained above. The proof if the rigid anchor plate assumption is valid is not carried out by PROFIS Engineering. Input data and results must be checked for agreement with the existing conditions and for plausibility!
- Checking the transfer of loads into the base material is required in accordance with ETAG 001, Annex C(2010)Section 7! The software considers that the grout is installed under the anchor plate without creating air voids and before application of the loads.
- The design is only valid if the clearance hole in the fixture is not larger than the value given in Table 4.1 of ETAG 001, Annex C! For larger diameters of the clearance hole see Chapter 1.1. of ETAG 001, Annex C!
- The accessory list in this report is for the information of the user only. In any case, the instructions for use provided with the product have to be followed to ensure a proper installation.
- The characteristic bond resistances depend on the return period (service life in years): 50

Fastening meets the design criteria!

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7 Installation data

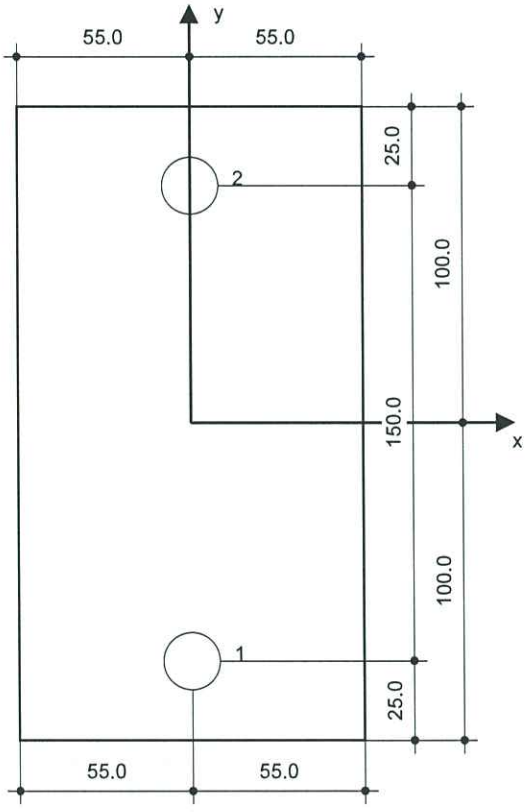
Anchor plate, steel: GB Q390; E = 205,000.00 N/mm^2; f_{yk} = 350.00 N/mm^2
Profile: no profile
Hole diameter in the fixture: d_f = 18.0 mm
Plate thickness (input): 15.0 mm
Recommended plate thickness: not calculated
Drilling method: Hammer drilled
Cleaning: Manual cleaning of the drilled hole according to instructions for use is required.

Anchor type and diameter: HST3-R M16 hef2
Item number: 2105876 HST3-R M16x135 35/15
Maximum installation torque: 110 Nm
Hole diameter in the base material: 16.0 mm
Hole depth in the base material: 106.0 mm
Minimum thickness of the base material: 160.0 mm

Hilti HST3 stud anchor with 98 mm embedment, M16 hef2, Stainless steel, installation per ETA-98/0001

7.1 Recommended accessories

Drilling	Cleaning	Setting
• Suitable Rotary Hammer • Properly sized drill bit	• Manual blow-out pump	• Torque wrench • Hammer



Coordinates Anchor [mm]

Anchor	x	y	c_{-x}	c_{+x}	c_{-y}	c_{+y}
1	0.0	-75.0	300.0	300.0	-	-
2	0.0	75.0	300.0	300.0	-	-

Input data and results must be checked for conformity with the existing conditions and for plausibility!
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美特鋁質有限公司

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發件人：馬 收件人：Wocky 日期：14-9-2022

工程 J858- LP-11 日出康成11 期

位置	T 1 - (1A)	樓層 9 /F	預留碼/絲桿 類型編號：CA04. CA06																附碼位置圖 1 張					備註
單位	預留碼位編號	1	2	3	4	5	6	7	8	9	10													田地收填滿无法量数
A	W：絲桿中至中																							
	X：左/右																							
	Y：上/下																							
	Z：出/入																							
單位	預留碼位編號	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16							
B	W：絲桿中至中																							
	X：左/右	50→	46→	3→	←3	13→	←12	←2	←5	←7	←17	←15	15→	10→										
	Y：上/下	↓10	↓8	↓20	↓10	↓7	↑5	↓7	↓8	↑6	↑5	↑2	↓17	↓23										
	Z：出/入	←3	←3	←5	←7	←3	3→	←4	←5	←6	←5	←3	5→	7→										
單位	預留碼位編號	1	2	3	4	5	6	7	8	9														
C	W：絲桿中至中																							
	X：左/右	24→	24→	50→	25→	25→	41→																	
	Y：上/下	↓14	↓23	↓13	↓13	↓22	↓23																	
	Z：出/入	←9	←15	←12	2→	2→	3→																	
單位	預留碼位編號	1	2	3	4	5	6	7	8	9														
D	W：絲桿中至中																							
	X：左/右	17→	12→	←5	←7	←32	←10																	
	Y：上/下	↓12	↓15	↓10	↓10	↓24	↓27																	
	Z：出/入	10→	←7	←8	←16	←8	←9																	

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工程 J858- LP-11 日出康成11 期 發件人：馬, 梁 收件人：Woody 日期：14-9-2022

位置	T 1 - (1A)	樓層 9 /F	預留碼/絲桿 類型編號： CA04. CA06	附碼位置圖 1 張	備註

單位	預留碼位編號	1	2	3	4	5	6	7	8	9	10	11	12	13
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	X：左/右	3→	2→	←2	1→	1→	2→	2→	←2	8→	←8			
	Y：上/下	↓10	↓20	↑5	↑3	↓10	↓11	↓9	↓11	↑3	↑2			
	Z：出/入	5→	←2	←20	9→	←6	←5	←3	←5	←3	←4			

單位	預留碼位編號	1	2	3	4	5	6	7	8	9	10	11	12	13
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	Y：上/下	↑10	↑10	↑13	↓30	↓52	↓60	↓64	↓54					
	Z：出/入	20→	10→	5→	5→	←10	←18	←15	←20					

單位	預留碼位編號	1	2	3	4	5	6	7	8	9
G	W：絲桿中至中									
	X：左/右	←28	←15	←25	←3	3→	15→			
	Y：上/下	↓60	↓34	↓34	↑7	↑15	↓8			
	Z：出/入	←4	←12	←8	3→	5→	8→			

備註

1 方格內數目字是偏離尺寸(以mm計)

2 碼位置偏號室外看每個單位由左至右排列

3. "0" 數目字表示留碼正確沒有偏差

4 X=欠留碼

符號說明

W：絲桿中至中 距離 +/--

X：預留碼←偏左 偏右→

Y：預留碼↑偏上 偏下↓

Z：預留碼←偏出 偏入→

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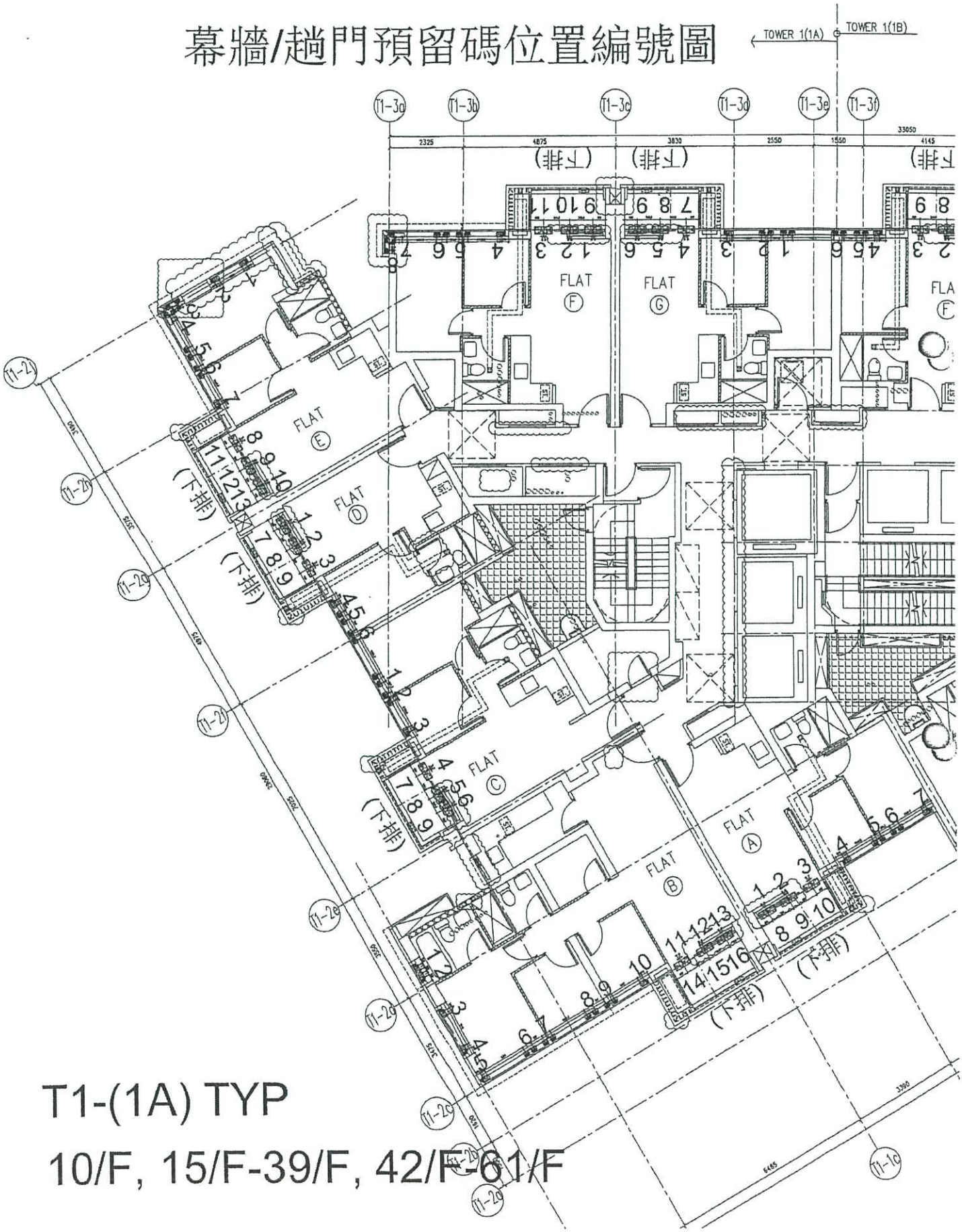
幕牆/趟門預留碼位置編號圖



T1-(1A) TYP

10/F, 15/F-39/F, 42/F-61/F

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工程 J858- LP-11 日出康成11 期 發件人：[illegible] 收件人：Woody 日期：14-9-2022

位置	T 1 - (1B)	樓層 9 /F				預留碼/絲桿 類型編號：CA04. CA06												附碼位置圖 1 張					備註
單位	預留碼位編號	1	2	3	4	5	6	7	8	9	10												
A	W：絲桿中至中																						天線位求 落石矢
	X：左/右					200→	225→	170→															
	Y：上/下					↓50	↓45	↓95															
	Z：出/入					←5	←10	←7															
單位	預留碼位編號	1	2	3	4	5	6	7	8														
B	W：絲桿中至中																						
	X：左/右	3→	←44	15→	←15	30→																	
	Y：上/下	↓10	↓5	↑3	↑7	↓15																	
	Z：出/入	←5	←7	←7	←8	←10																	
單位	預留碼位編號	1	2	3	4	5	6	7	8	9	10	11											
C	W：絲桿中至中																						
	X：左/右	←50	←40	5→	←5	←15	←50	←44	40→														
	Y：上/下	↑73	↑70	↑70	↑75	↑75	↑44	↑50	↑27														
	Z：出/入	5→	4→	8→	6→	5→	8→	7→	←3														
單位	預留碼位編號	1	2	3	4	5	6	7	8	9	10	11	12	13									
D	W：絲桿中至中																						
	X：左/右	40→	←70	←60	←45	←180	←30	←90	←45	←50	←60												
	Y：上/下	↑15	↑10	↑15	↑15	↑10	↓5	↓6	↓18	↓20	↓7												
	Z：出/入	←40	←9	←7	←15	←20	←20	←30	←13	←12	←12												

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位置	T 1 - (1B) 樓層 9 /F	預留碼/絲桿 類型編號：CA04. CA06.															附碼位置圖				張	備註
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	Y：上/下	↓30	↓25	↓7	↓10	↑5																
	Z：出/入	8→	4→	10→	5→	20→																
單位	預留碼位編號	1	2	3	4	5	6	7	8													
F	W：絲桿中至中																					
	X：左/右	15→	15→	20→	←3	←35	←45															
	Y：上/下	↓10	↓13	↓11	↑8	↑3	↑20															
	Z：出/入	17→	11→	8→	4→	9→	10→															

備註

1 方格內數目字是偏離尺寸(以mm計)

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4 X=欠留碼

符號說明

W：絲桿中至中 距離 + / --

X：預留碼←偏左 偏右→

Y：預留碼↑偏上 偏下↓

Z：預留碼←偏出 偏入→

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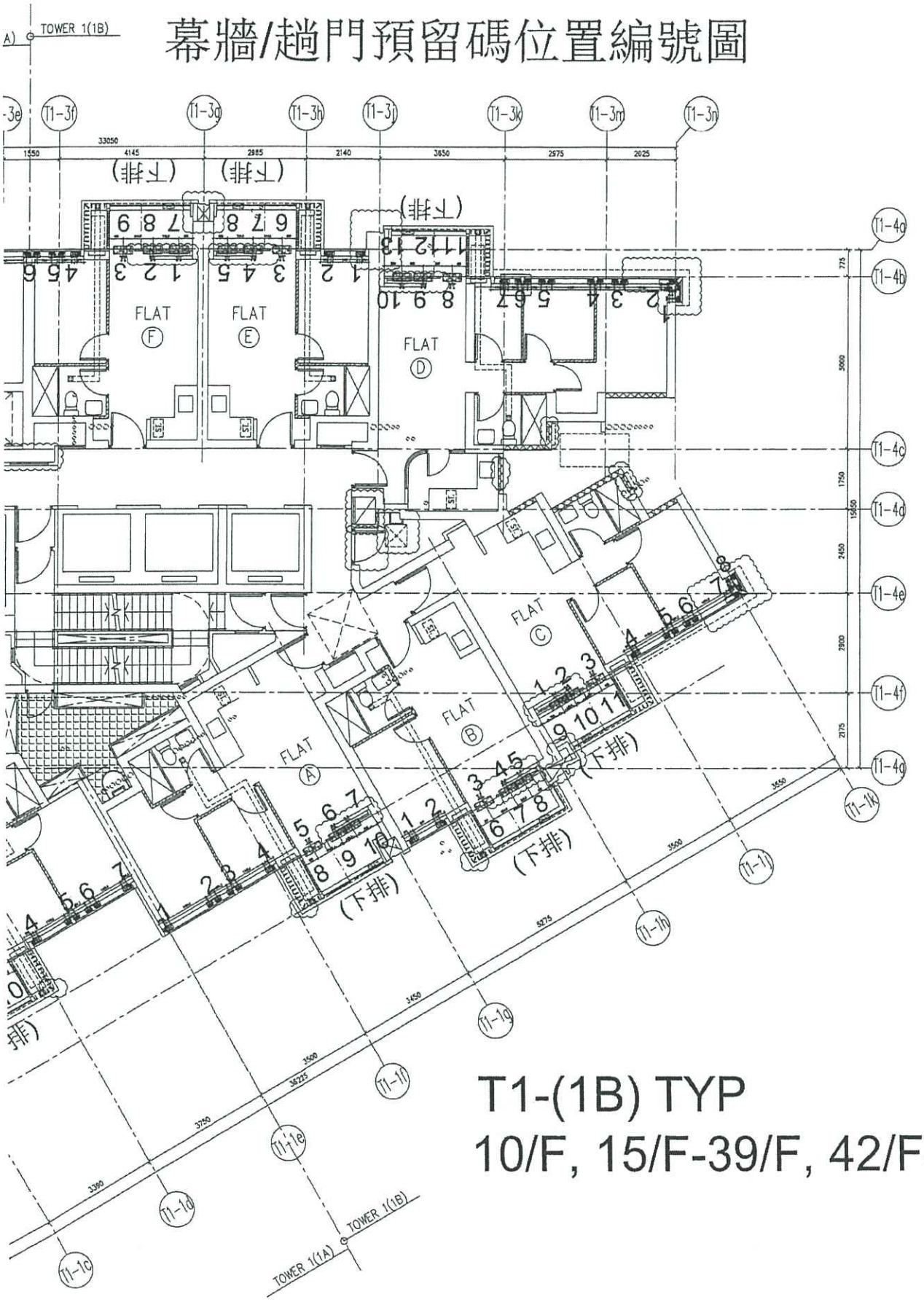
幕牆/趟門預留碼位置編號圖



T1-(1B) TYP

10/F, 15/F-39/F, 42/F-61/F